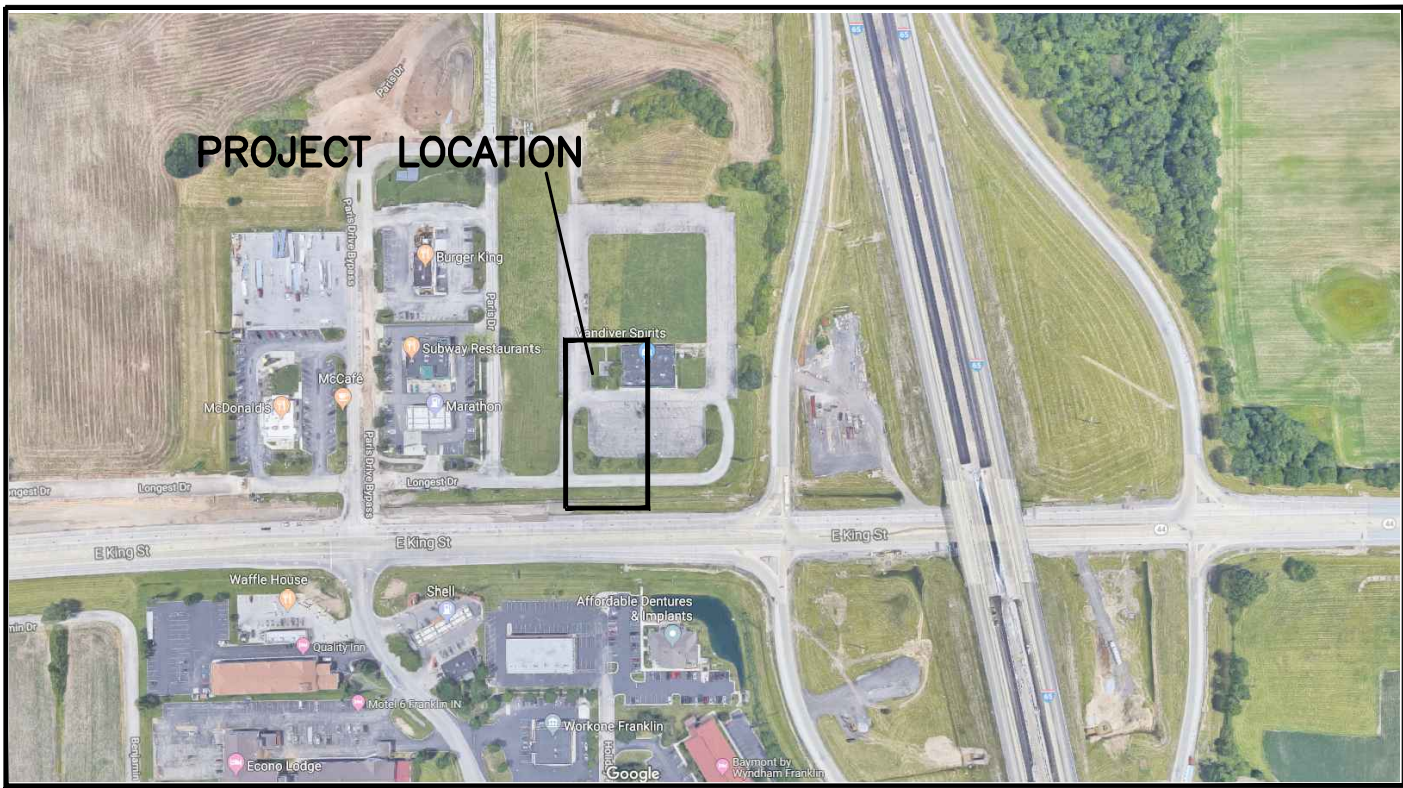
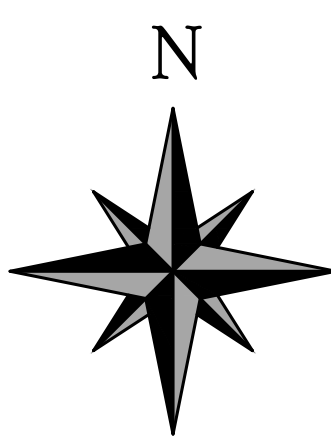
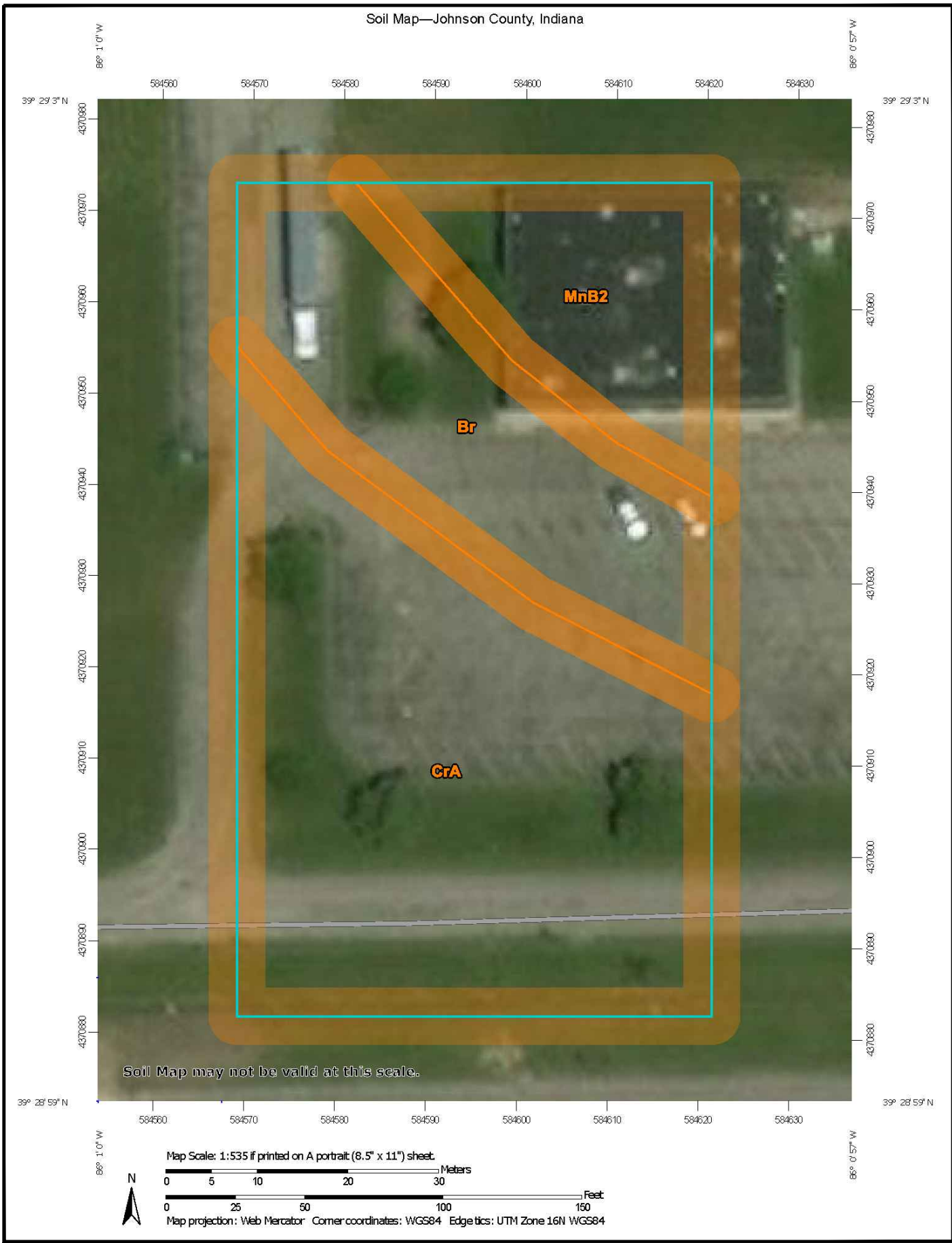




VICINITY MAP



LOCATION MAP



Starbucks

Lot 3, Franklin Gateway Development, Section 2

Franklin, Indiana 46131

OWNER INFORMATION

FRANKLIN RETAIL LLC
6440 WESTFIELD BLVD.
INDIANAPOLIS, INDIANA
PHONE: (317) 472-1800

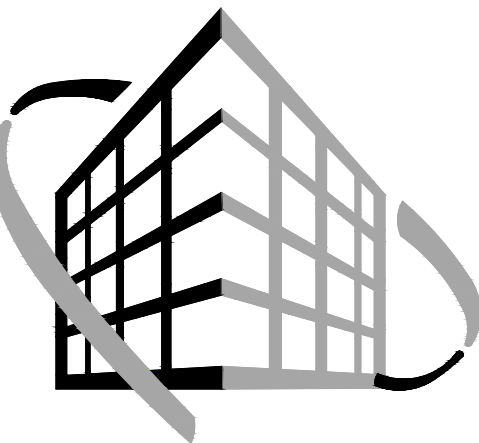
BUILDING ADDRESS INFORMATION:

STARBUCKS
FRANKLIN GATEWAY DEVELOPMENT
FRANKLIN, INDIANA 46131

BUILDING INFORMATION:

OCCUPANCY GROUP: A-2 CONSTRUCTION TYPE: V-B (NON-SPRINKLED)
BUILDING INFORMATION - TOTAL SQUARE FOOTAGE: 2,255.00 sq.ft.
BUILDING MAX. HT. 26'-2" A.F.F.
BUILDING DESIGN TO MEET EARTHQUAKE ZONE ONE
BUILDING TO MEET ADA (AMERICAN DISABILITY ACT)

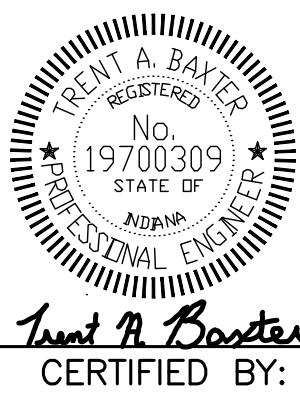
ENGINEERING AND CERTIFICATION:



**VERSATILE
CONSTRUCTION
GROUP, LLC.**

570 East Tracy Road, Suite 610
New Whiteland, Indiana 46184
Ph: 317.535.3579 Fax: 317.535.3581

PLANS CERTIFIED BY:
TRENT A. BAXTER P.E.
REGISTERED P.E. No. 19700309
DATE: January 21, 2019



UTILITY COMPANIES:

ELECTRICAL UTILITY:
DUKE ENERGY
2515 N. MORTON ST.
FRANKLIN, INDIANA 46131
PHONE: (317) 736-2017

FIRE DEPARTMENT:
CITY OF FRANKLIN FIRE DEPT.
1800 THORNBURG LANE
FRANKLIN, INDIANA 46131
PHONE: (317) 736-3650

GAS UTILITY:
VECTREN GAS COMPANY
600 INDUSTRIAL DRIVE
FRANKLIN, INDIANA 46131
PHONE: (317) 736-2907

WATER UTILITY:
INDIANA AMERICAN WATER COMPANY
425 W. MAIN STREET
MOORESVILLE, INDIANA 46158
PHONE: (317) 831-3385

SANITARY SEWER:
CITY OF FRANKLIN DPW
796 S. STATE STREET
FRANKLIN, INDIANA 46131
PHONE: (317) 736-3648

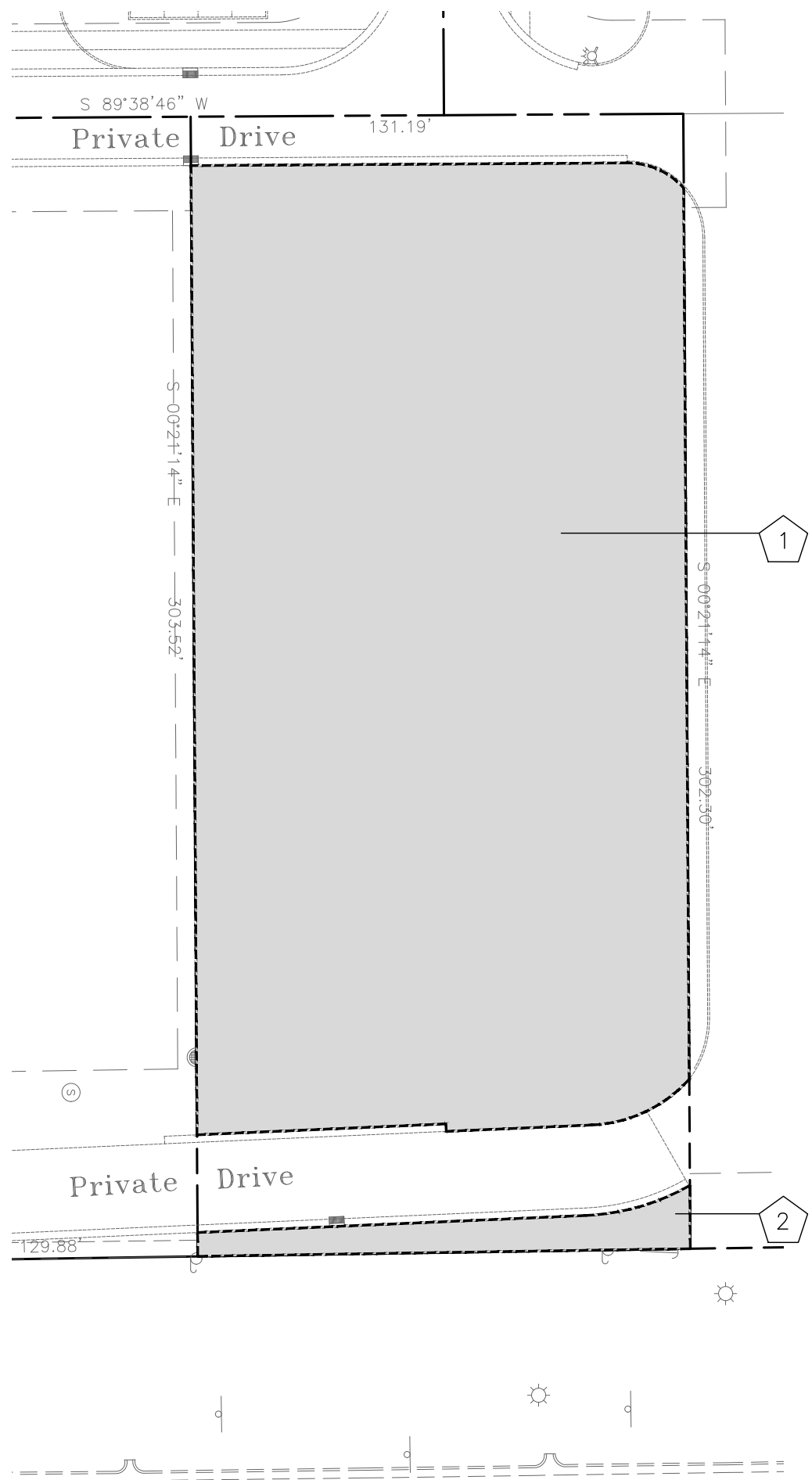
COMMUNICATION:
METRO FIBERNET
8036 COLE WOOD BLVD.
INDIANAPOLIS, INDIANA 46239
PHONE: (317) 809-8067

FIBEROPTIC:
CENTURY LINK
1147 N. MORTON STREET
FRANKLIN, INDIANA 46131
PHONE: (317) 736-4863

COMMUNICATION:
COMCAST
1600 W. VERNAL PIKE
BLOOMINGTON, INDIANA 47404
PHONE: (812) 360-3090

INDEX TO DRAWINGS

SHEET NO.	DESCRIPTION
T100	TITLE SHEET
CIVIL (SITE) PLANS	
C010	SITE AREA AND ZONING PLAN
C100	EXISTING SITE CONDITION PLAN
C200	SITE LAYOUT PLAN
C210	SITE LANDSCAPING PLAN
C300	SITE UTILITY PLAN
C310	SITE PHOTOMETRIC PLAN
C400	SITE GRADING PLAN
C500	SITE EROSION CONTROL PLAN
C510	SITE EROSION CONTROL DETAILS
C520	SITE EROSION CONTROL DETAILS
C530	EROSION CONTROL DETAILS, IMPLEMENTATION AND MAINTENANCE GUIDELINES
C600	GENERAL DETAILS
C610	GENERAL DETAILS
C620	GENERAL DETAILS
C630	GENERAL DETAILS
C640	GENERAL DETAILS
C650	GENERAL DETAILS
C700	GENERAL NOTES

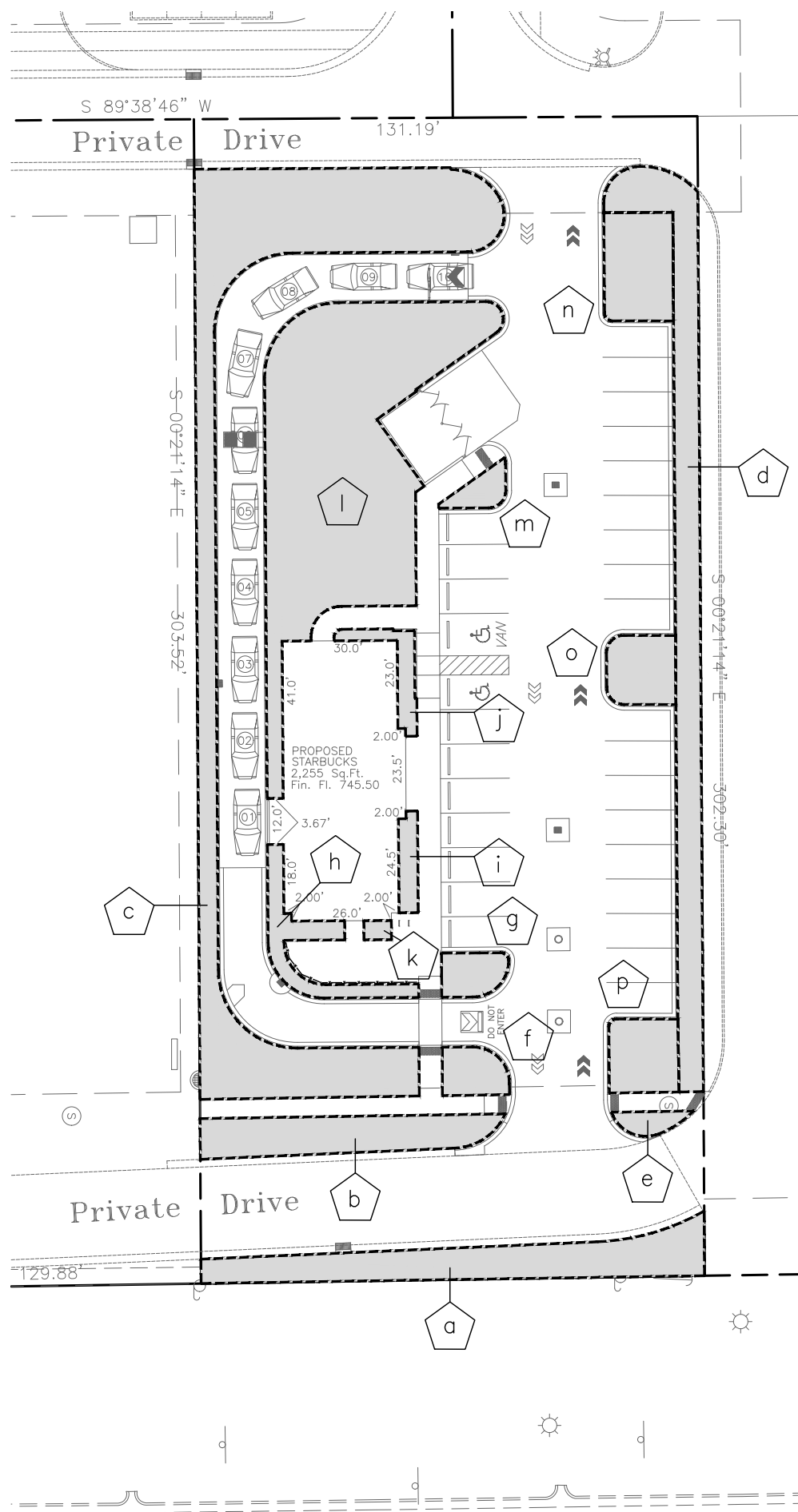


E. KING ST. (S.R. 44)

EXISTING SITE AREA KEY PLAN
SCALE: 1"= 40.0'
(SEE SHEET No.s C100 FOR SPECIFIC INFORMATION)

GENERAL PROPOSED SITE INFORMATION:

DESCRIPTIONS:	AREAS:
TOTAL LOT AREA:	0.912 acres
BUILDING AREAS:	
PROPOSED RETAIL CENTER	2,255.0 sq.ft.
TOTAL PARKING:	
PARKING SPACES REQUIRED: (REQUIREMENT: 1 / 300 sq.ft.)	08 SPACES
PARKING SPACES PROVIDED:	30 SPACES
BUILDING SET BACKS:	
NORTH (REAR YARD):	20.0'
SOUTH (FRONT YARD):	30.0'
EAST (SIDE YARD):	15.0'
WEST (SIDE YARD):	15.0'
ZONING:	
PROPERTY ZONING:	MXR
NORTH ZONING:	MXR
SOUTH ZONING:	MXR
EAST ZONING:	MXR
WEST ZONING:	MXR



E. KING ST. (S.R. 44)

PROPOSED SITE AREA KEY PLAN
SCALE: 1"= 40.0'
(SEE SHEET No.s C200 & C210 FOR SPECIFIC LAYOUT INFORMATION)

LEGAL DESCRIPTION:

A PART OF THE SOUTH HALF OF SECTION 18, TOWNSHIP 12 NORTH, RANGE 5 EAST, BEING A LAND DESCRIPTION PREPARED FROM A SURVEY TITLED "FRANKLIN GATEWAY DEVELOPMENT BOUNDARY SURVEY" PERFORMED BY INDEPENDENT LAND SURVEYING, INC (JOB#17174), CERTIFIED BY TIM M. ALLEN PLS (LS-20700102) ON AUGUST 10, 2018, INTENDED TO BE THE PROPOSED LOT 3 IN "FRANKLIN GATEWAY DEVELOPMENT SECTION 2", BEING A PORTION OF THAT LAND DESCRIBED AND RECORDED IN THE FOLLOWING (INST.#2017-029600, INST.#2017-025086, & INST.#2001-006116) IN THE OFFICE OF THE RECORDER OF JOHNSON COUNTY INDIANA; AND DESCRIBED AS FOLLOWS:

COMMENCING AT A 5/8" CAPPED REBAR (FOUND) MARKING THE NORTHWEST CORNER OF THE SOUTHEAST QUARTER OF SAID SECTION; THENCE NORTH 89°41'46" EAST (AN ASSUMED BEARING) ALONG THE NORTH LINE OF SAID QUARTER A DISTANCE OF 50.50 FEET TO THE EAST RIGHT-OF-WAY LINE OF PARIS DRIVE AND THE NORTHWEST CORNER OF "J ENTERPRISES INN OF NASHVILLE, LLC." (INST.#2018-019410); THENCE SOUTH 00°21'14" EAST ALONG SAID RIGHT-OF-WAY, AND THE WEST LINES OF "J ENTERPRISES INN OF NASHVILLE, LLC.", "J ENTERPRISES INN OF NASHVILLE, LLC." (INST.#2017-016364), "PAD COMMERCIAL, LLC." (INST.#2017-029600), & "CITY OF FRANKLIN" (INST. 2017-025086) A DISTANCE OF 1,110.33 FEET TO A 5/8" REBAR AT THE SOUTHWEST CORNER OF "CITY OF FRANKLIN" AND THE NORTH RIGHT-OF-WAY LINE OF STATE ROAD 44; THENCE NORTH 89°06'46" EAST ALONG SAID RIGHT-OF-WAY AND "CITY OF FRANKLIN" PASSING THROUGH A 5/8" REBAR (FOUND) AT THE SOUTHWEST CORNER OF "EAST KING STREET LLC." (INST. 2001-006116) AT 126.77 FEET A TOTAL DISTANCE OF 129.88 FEET TO THE POINT OF BEGINNING; THENCE CONTINUING NORTH 89°06'46" EAST ALONG SAID RIGHT-OF-WAY AND "CITY OF FRANKLIN" A DISTANCE OF 131.20 FEET TO A POINT; THENCE NORTH 00°21'41" WEST A DISTANCE OF 302.30 FEET TO A POINT; THENCE SOUTH 89°38'46" WEST A DISTANCE OF 131.19 FEET TO A POINT; THENCE SOUTH 00°21'14" EAST A DISTANCE OF 303.52 FEET TO THE POINT OF BEGINNING, CONTAINING 0.91 ACRES, MORE OR LESS, AND SUBJECT TO ALL LEGAL RIGHTS OF WAY AND EASEMENTS.

EXISTING LANDSCAPING/ UNDEVELOPED AREAS:

SYMBOLS:	DESCRIPTIONS:	AREAS:
1	EXISTING LANDSCAPING AREA:	33,552.09 sq.ft.
2	EXISTING LANDSCAPING AREA:	1,138.78 sq.ft.
TOTAL EXISTING LANDSCAPING/ UNDEVELOPED AREA (87.3%):		34,690.87 sq.ft.

PROPOSED LANDSCAPING AREAS:

SYMBOLS:	DESCRIPTIONS:	AREAS:
a	PERIMETER LANDSCAPING AREA:	1,138.78 sq.ft.
b	PERIMETER LANDSCAPING AREA:	724.64 sq.ft.
c	PERIMETER LANDSCAPING AREA:	3,668.12 sq.ft.
d	PERIMETER LANDSCAPING AREA:	1,739.66 sq.ft.
e	PERIMETER LANDSCAPING AREA:	93.38 sq.ft.
f	INTERIOR LANDSCAPING AREA:	199.80 sq.ft.
g	INTERIOR LANDSCAPING AREA:	181.06 sq.ft.
h	INTERIOR LANDSCAPING AREA:	353.56 sq.ft.
i	INTERIOR LANDSCAPING AREA:	128.52 sq.ft.
j	INTERIOR LANDSCAPING AREA:	172.56 sq.ft.
k	INTERIOR LANDSCAPING AREA:	35.83 sq.ft.
l	INTERIOR LANDSCAPING AREA:	3,548.75 sq.ft.
m	INTERIOR LANDSCAPING AREA:	120.66 sq.ft.
n	INTERIOR LANDSCAPING AREA:	507.73 sq.ft.
o	INTERIOR LANDSCAPING AREA:	315.31 sq.ft.
p	INTERIOR LANDSCAPING AREA:	345.78 sq.ft.
TOTAL LANDSCAPING AREA (38.3%):		13,274.13 sq.ft.

PROPOSED LANDSCAPING CODE REQUIREMENTS:

TOTAL IMPERVIOUS SURFACE:
(NOT TO EXCEED 75% OF LOT COVERAGE)
ACTUAL COVERAGE: 0.607± ACRES (66.6%)

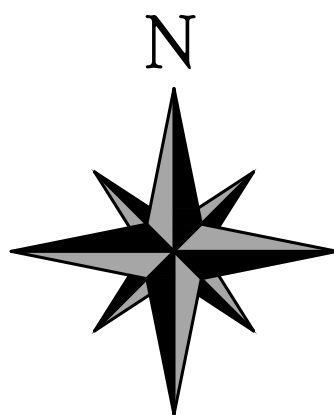
PAVING SURFACE:
(NOT INCLUDING BUILDING OR SIDEWALK)
21,407.70 sq.ft.

INTERIOR LANDSCAPE REQUIREMENT:
(5% MINIMUM OF THE IMPERVIOUS SURFACE, MINIMUM 300 SQ.FT. ISLAND)
ACTUAL LANDSCAPING: 5,909.56 sq.ft. (14.9%)



UTILITY DISCLAIMER

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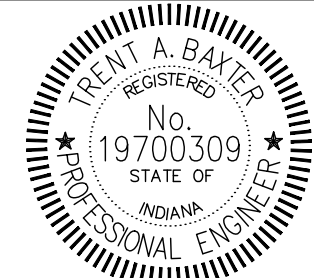
570 East Tracy Road, Suite 610
New Whiteland, Indiana 46184
Ph: 317.535.3579 Fax: 317.535.3581

Starbucks
Franklin Gateway Development
Franklin, Indiana

Site Area and Zoning Information Plan

Job No.:	Date Stamped:
17023	01/21/2019
Drawn By:	Checked By: Scale:
caw	tab 1"= 20.0'

CAD FILE:
G:\17023\010 site zoning and area plan.dwg



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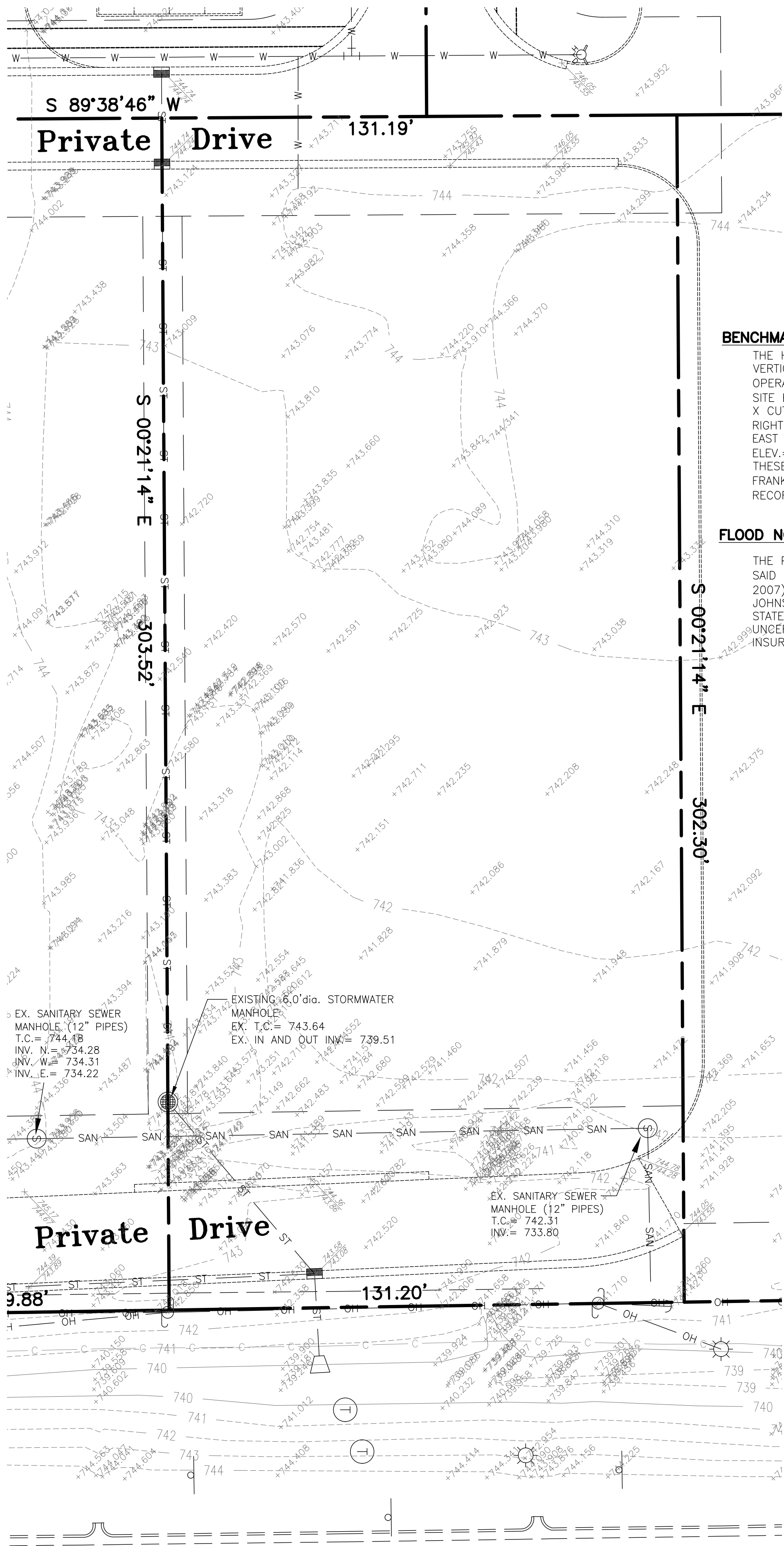
SHEET TITLE:

C010

LEGAL DESCRIPTION:

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- LEGEND:
- EXISTING SPOT ELEVATION
 - EXISTING CONTOURS LINES
 - EXISTING WATER SERVICE LINE
 - EXISTING SANITARY SEWER SERVICE LINE
 - EXISTING OVERHEAD UTILITY LINE
 - EXISTING POWER POLES
 - EXISTING GAS LINES
 - EXISTING STORM SEWER LINES
 - EXISTING STORM SEWER INLETS

BENCHMARKS:
THE HORIZONTAL COORDINATE SYSTEM IS NAD 83 INDIANA EAST 1301 AND VERTICAL DATUM IS NAD88 BASED ON THE INDIANA CONTINUOUSLY OPERATING (INCOR).
SITE BENCHMARK (TBM #1)
X CUT ON SE BONNETT BOLT OF FIRE HYDRANT LOCATE ALONG EAST RIGHT OF WAY LINE OF GATEWAY DRIVE. 60' +/- NORTH AND 1' +/- EAST OF SW CORNER OF LOT 2.
ELEV.= 746.69 (NAVD 88).
THESE ELEVATIONS WERE USED BY INDEPENDENT LAND SURVEYING FOR FRANKLIN GATEWAY DEVELOPMENT SECONDARY PLAT- SECTION 2 (NOT RECORDING AT THE TIME OF TOPOGRAPHICAL SURVEY).

FLOOD NOTE:
THE PARCEL DESCRIBED AND SHOWN HEREIN LIES WITHIN ZONE "X" AS SAID PARCEL PLOTS ON MAP NUMBER 18081002320 (DATED AUGUST 2, 2007) OF THE FLOOD INSURANCE RATE MAPS FOR THE CITY OF FRANKLIN, JOHNSON COUNTY, INDIANA. THE ACCURACY OF THIS FLOOD HAZARD STATEMENT IS SUBJECT TO MAP SCALE UNCERTAINTY AND TO ANY OTHER UNCERTAINTY IN LOCATION OR ELEVATION ON THE REFERENCED FLOOD INSURANCE RATE MAP.

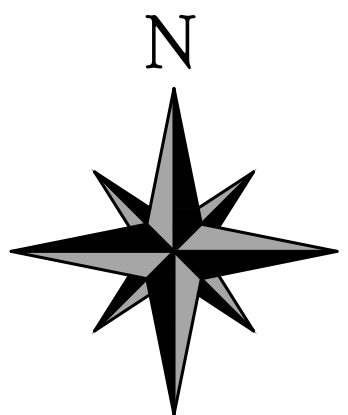
EROSION CONTROL INFORMATION:
SEE SITE EROSION CONTROL PLAN FOR LOCATIONS AND INSTALLATION OF ALL EROSION CONTROL MEASURES REQUIRED ON THIS SITE.

EXISTING SITE CONDITION PLAN
SCALE: 1"= 20.0'



UTILITY DISCLAIMER

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CONSTRUCTION
GROUP, LLC.

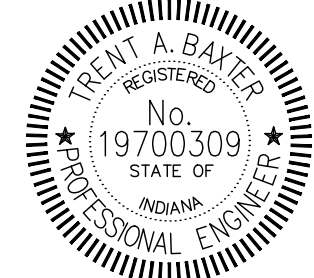
570 East Tracy Road, Suite 610
New Whiteland, Indiana 46184
Ph: 317.535.3579 Fax: 317.535.3581

Starbucks
Franklin Gateway Development
Franklin, Indiana

Existing Site Condition Plan

Job No.:	Date Stamped:
17023	01/21/2019
Drawn By:	Checked By: Scale:
caw	tab 1"= 20.0'

CAD FILE:
G:\17023\c100 existing site condition plan.dwg



Trent A. Baxter
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SHEET TITLE:

C100

E. KING ST. (S.R. 44)

C200

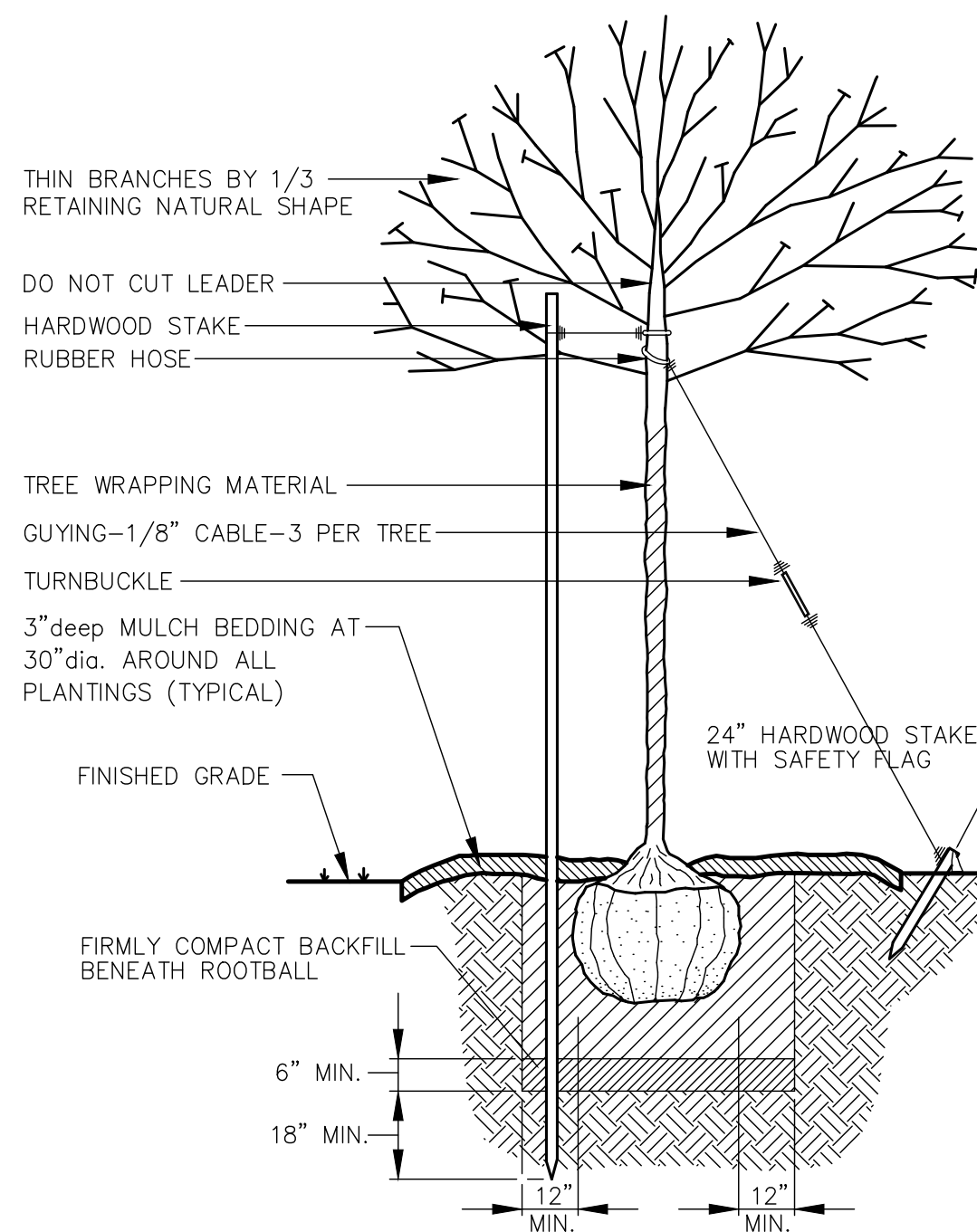
PLANTING SPECIFICATIONS:

- ALL PLANT MATERIALS SHALL CONFORM TO THE STANDARDS SET FORTH IN THE CURRENT EDITION OF THE AMERICAN STANDARDS NURSERY STOCK OF NURSERYMEN. PLANTS SHALL BE TYPICAL OF SPECIES AND VARIETY, AND HAVE NORMAL, WELL-DEVELOPED BRANCHING STRUCTURE AND VIGOROUS FIBROUS ROOT SYSTEM.
- PLANTS SHALL BE HEALTHY, VIGOROUS PLANTS FREE FROM INSECTS AND DISEASE. TRUNK AND STEMS SHALL BE FIRM WITH NO INDICATION OF FUNGUS CANKERS OR GALLS, INSECT BORERS, DIEBACK, FRONT CRACKS, OR OTHER DEFECTS.
- ALL PLANTS SHALL BE COMMERCIAL GROWN AND NO PLANTS FROM THE WILD SHALL BE ACCEPTABLE WITHOUT SPECIFIC APPROVAL FROM THE DIVISION OF BUILDING INSPECTION.
- TREES SHALL NOT BE ACCEPTABLE IF THEIR CENTRAL LEADER HAS BEEN CUT OR IS DAMAGED SO THAT CUTTING IS NECESSARY.
- PLANTS SHALL NOT BE PRUNED PRIOR TO INSTALLATION. ANY NECESSARY PRUNING SHALL BE DONE IMMEDIATELY AFTER THE TIME OF INSTALLATION.
- PLANT HEIGHT SHALL BE MEASURED BEFORE PRUNING WITH BRANCHES IN A NORMAL POSITION. NO PLANT SHALL BE PRUNED BACK TO SUCH AN EXTENT THAT IT NO LONGER MEETS THE REQUIRED SIZE SPECIFICATIONS.
- ALL DECIDUOUS AND EVERGREEN TREES SHALL BE BALLED AND BURLAPPED. NO BARE ROOT TREES SHALL BE ACCEPTABLE.
- THE MINIMUM SIZE OF BALLS, BALL DEPTHS, AND BALL DIAMETERS SHALL BE IN ACCORDANCE WITH RECOMMENDED BALLING AND BURLAPPING SPECIFICATIONS AS SET FORTH IN THE CURRENT EDITION OF THE AMERICAN STANDARDS OF NURSERY STOCK.
- ALL BALLED AND BURLAPPED PLANTS WHICH CANNOT BE PLANTED IMMEDIATELY SHALL BE SEALED IN AND PROTECTED WITH BURLAP OR OTHER ACCEPTED MATERIAL.
- IDEAL TREES SELECTED FOR THIS PROJECT HAVE A STRAIGHT TRUNK, ROUNDED OR OVAL FORM, AND SYMMETRICAL BRANCHING PATTERN.
- ALL TREES AND SHRUBS SHOWN ON THIS DRAWING SHALL BE PLACED IN A MANNER TO ALLOW FOR AMPLE ROOM TO DEVELOP.
- IF WORK IS REQUIRED WITHIN THE EASEMENTS CAUSING REMOVAL OR DAMAGE OF LANDSCAPE MATERIALS, THE PROPERTY OWNER SHALL BE RESPONSIBLE FOR REPLACEMENT OF MATERIALS ACCORDING TO THE APPROVED LANDSCAPE PLAN.
- ALL LANDSCAPED AREAS SHALL BE NON-COMPACTED SOIL TO A MINIMUM DEPTH OF 24" UNLESS OTHERWISE NOTED.
- ALL SHRUB MASSES ALONG FRONT PARKING AREA SHALL BE ALLOWED TO FORM A CONTINUOUS SCREENING AND SHALL NOT BE TRIMMED INTO INDIVIDUAL SHRUBS.

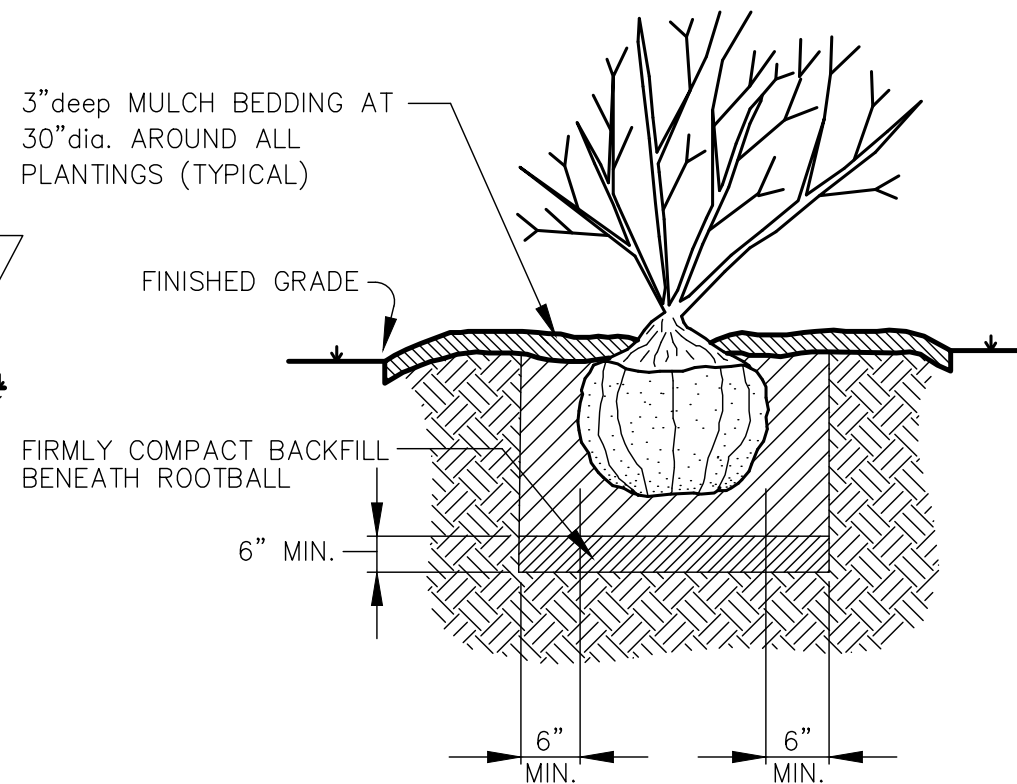
Landscaping Notes

- All plant materials to meet minimum standards set by American Association of Nurserymen, latest published addition of American Standard for Nursery Stock.
- All areas disturbed by construction and not otherwise paved or landscaped and all lawn areas shall be seeded with straw as specified.
- All lawn areas to be raked smooth, fertilized and watered prior to seeding.

% of mix.	Name of grass	Application
39.86%	Falcon IV tall fescue	1.5 #/1,000 sq.ft.
29.80%	Scorpion II tall fescue	1.5 #/1,000 sq.ft.
29.60%	Six point tall fescue	1.5 #/1,000 sq.ft.
00.72%	Inert	1.5 #/1,000 sq.ft.
00.02%	Other weeds	1.5 #/1,000 sq.ft.
- Planting soil for tree and shrub pits to be backfilled with the soil that was removed from the planting pit.
- Any and all planting material substitutions shall be approved by the local greenspace administrator.



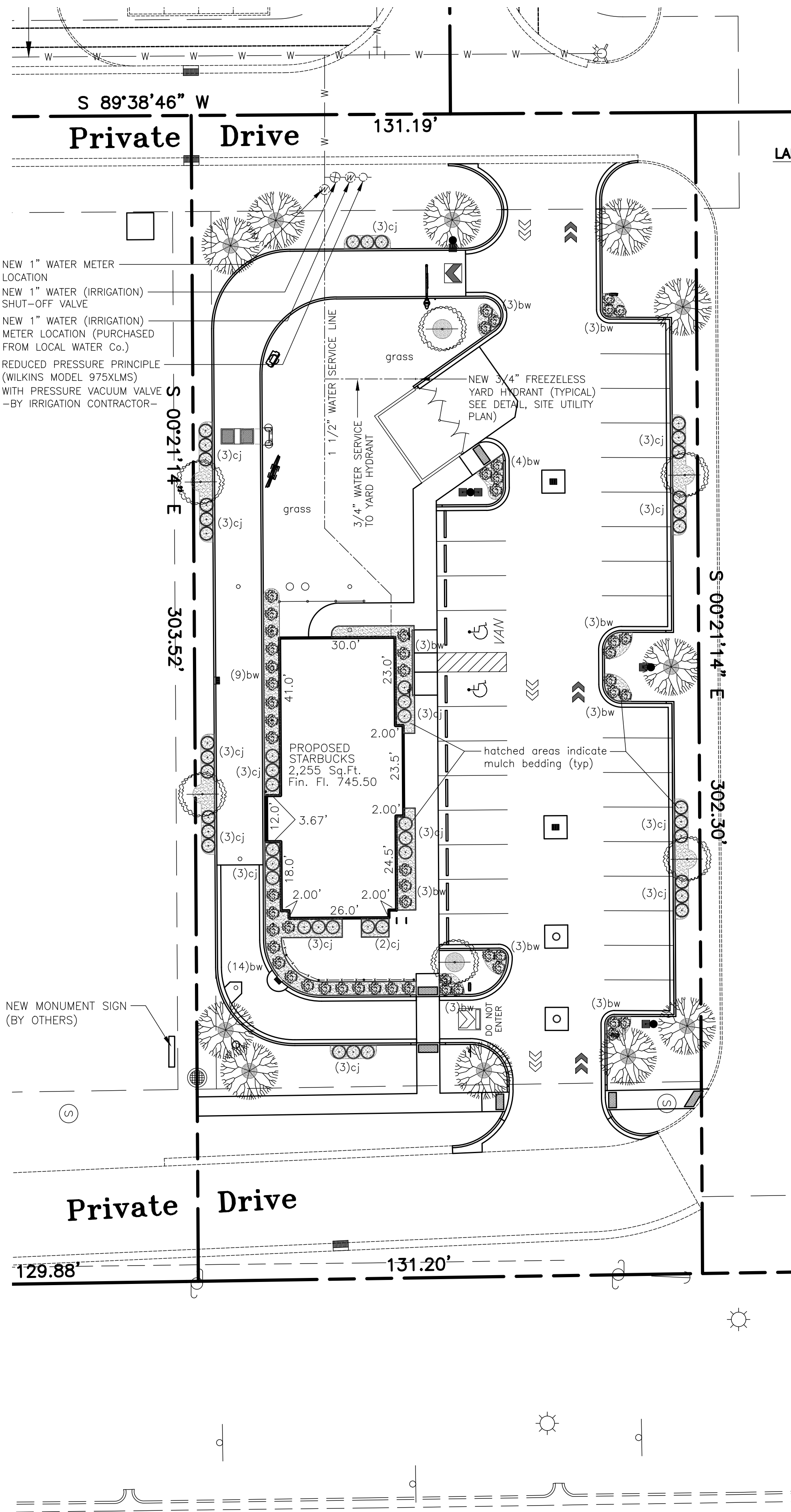
TREE PLANTING DETAIL
NO SCALE



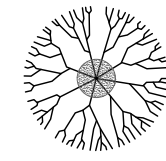
SHRUB AND TREE PLANTING NOTES:

- GROUND LINE TO BE THE SAME AS NURSERY. ROOTBALL RAISED 1 1/2" ABOVE FIN. GRADE TO ALLOW FOR SETTLING.
- CONSTRUCT 3"deep MULCH RING AROUND ALL TREES & SHRUBS. FLOOD TWICE W/ IN 24 HRS.
- CUT AND REMOVE BURLAP FROM TOP OF ROOTBALL.

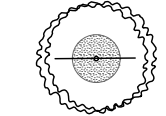
SHRUB PLANTING DETAIL
NO SCALE



LANDSCAPE LEGEND:



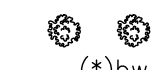
DECIDUOUS SHADE TREE
BOTANICAL NAME: GINKGO BILOBA 'FAIRMOUNT'
COMMON NAME: MAIDENHAIR TREE (MALE)
CALIPER: 2 1/2" (single stem)
HEIGHT AT PLANTING: 7.0'tall
QUANTITY: 11
5.0'dia. MULCH BEDDING



EVERGREEN ORNAMENTAL TREE
BOTANICAL NAME: THUJA PLICATA
COMMON NAME: GREEN GIANT ARBORVITAE
CALIPER: 2" (single stem)
HEIGHT AT PLANTING: 5.0'tall (min.)
QUANTITY: 06
5.0'dia. MULCH BEDDING



DECIDUOUS SHRUB:
BOTANICAL NAME: BERBERIS THUNBERGII
COMMON NAME: CRIMSON PYGMY JAPANESE BARBERRY
HEIGHT AT PLANTING: 24" (MIN)
QUANTITY: 47
(MATURE HT.= 36"tall)



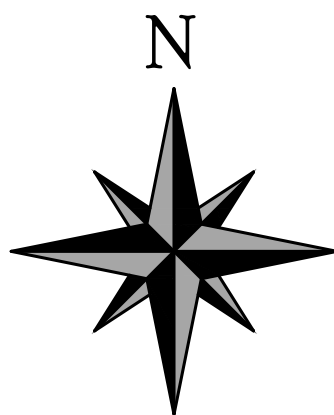
DECIDUOUS SHRUB:
BOTANICAL NAME: BUXUS SEMPERVIRENS
COMMON NAME: NORTH STAR BOXWOOD
HEIGHT AT PLANTING: 18" (MIN)
QUANTITY: 50
(MATURE HT.= 24"to 32"tall)

SITE LANDSCAPING PLAN
SCALE: 1"= 20.0'



UTILITY DISCLAIMER

EXISTING UNDERGROUND INSTALLATIONS SUCH AS WATER MAINS, GAS MAINS, SEWERS, TELEPHONE LINES, AND BURIED STRUCTURES IN THE VICINITY OF THE WORK TO BE DONE HEREUNDER ARE INDICATED ON THE DRAWINGS ONLY TO THE EXTENT SUCH INFORMATION HAS BEEN MADE AVAILABLE TO OR DISCOVERED BY THE SURVEYOR IN PREPARING THIS DRAWING. THERE IS NO GUARANTEE AS TO THE ACCURACY OR COMPLETENESS OF SUCH INFORMATION, AND ALL RESPONSIBILITY FOR ACCURACY AND COMPLETENESS THEREOF IS EXPRESSLY DISCLAIMED.



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CONSTRUCTION
GROUP, LLC.

570 East Tracy Road, Suite 610
New Whiteland, Indiana 46184
Ph: 317.535.3579 Fax: 317.535.3581

Starbucks
Franklin Gateway Development
Franklin, Indiana

Site Landscaping Plan

Job No.:

17023

Date Stamped:

01/21/2019

Drawn By:

caw

Checked By:

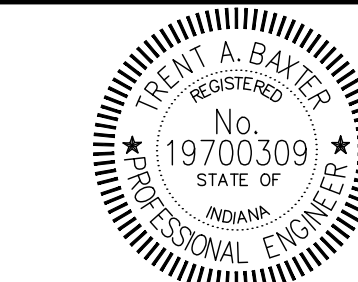
tab

Scale:

1"= 20.0'

CAD FILE:

c:\17023\c210 site landscaping plan.dwg



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SHEET TITLE:

C210

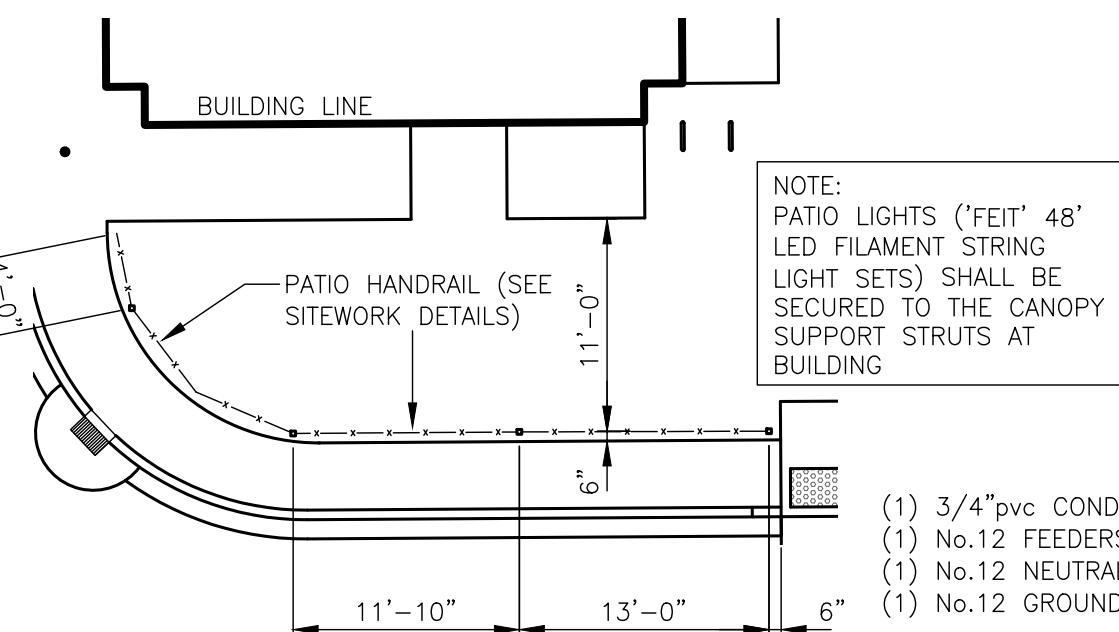
E. KING ST. (S.R. 44)

UTILITY CONSTRUCTION NOTES:

- Site Contractor shall have approval of all governing agencies having jurisdiction over these systems prior to any installations.
- The Sanitary Sewer pipe material shall be pvc unless otherwise noted. All pipes shall meet the ASTM 3034 with the wall thickness equal to SDR 35. The pipe shall be joined with rubber ring gaskets as specified in ASTM 3034 and shall be installed in accordance with ASTM 2321. Furthermore the pipe material cell classification shall be 12454B as per ASTM 1784.
- All trenching, pipe laying, and backfilling shall be in accordance with all federal OSHA regulations.
- The contractor shall be responsible for repairs to any existing utilities damaged during construction.
- The contractor shall refer to the Architect's plans and specifications for the actual location of all utility entrances to include sanitary sewer laterals, domestic and fire protection water services, electrical, telephone, and gas services. The contractor shall coordinate installation of utilities in such a manner as to avoid conflicts and assure proper depths are achieved, as well as coordinating with the regulatory agency as to the location and scheduling of all tie-ins/connections to said services.
- All vertical bends on the proposed water mains shall be restrained with a mechanical joint fittings supplied with retainer glands. Any joints 25 feet or less from either side of the vertical bends shall be restrained with a retainer gland.
- All dimensions shown here are to centerline of pipe, fitting, or structure.
- All valves shall be installed in a cast iron valve box with cover.
- Thrust blocking shall be provided at all horizontal bends, tees, and fire hydrants.
- The minimum cover depths of all water line/mains shall be 54" deep.
- All gate valves 3/4" through 3" shall be bronze 'wedge' type gate valves. All valves shall have non-rising stems with solid tee head operating nut.
- All pipes 3" in diameter and smaller shall be type K copper. All fittings shall be copper or cast bronze. All joints shall be solder or flare type joints.
- The minimum horizontal separation between the closest two points of the water service and sanitary sewer service lines is to be ten feet (10'). The minimum vertical separation between the closest two points of the water service and sanitary sewer service lines is twenty-four inches (24").
- All water mains shall be hydrostatically tested and disinfected before acceptance, as per AWWA standard specifications.

SANITARY SEWER PIPING INSTALLATION:

- 22.8 L.F. OF 6"pvc SANITARY SEWER LATERAL AT 2.05% SLOPE
 - 1,000 GALLON GREASE SEPARATOR (SEE SITEWORK DETAILS)
inv. in= 741.53
inv. out= 741.28
 - 11.3 L.F. OF 6"pvc SANITARY SEWER LATERAL AT 23.54% SLOPE
 - 10.8 L.F. OF 6"pvc SANITARY SEWER LATERAL AT 2.96% SLOPE
 - 148.8 L.F. OF 6"pvc SANITARY SEWER LATERAL AT 2.05% SLOPE
- NOTE:
ALL SANITARY SEWER LATERAL PIPING SHALL HAVE A 10ga. INSULATED TRACER WIRE (typical)



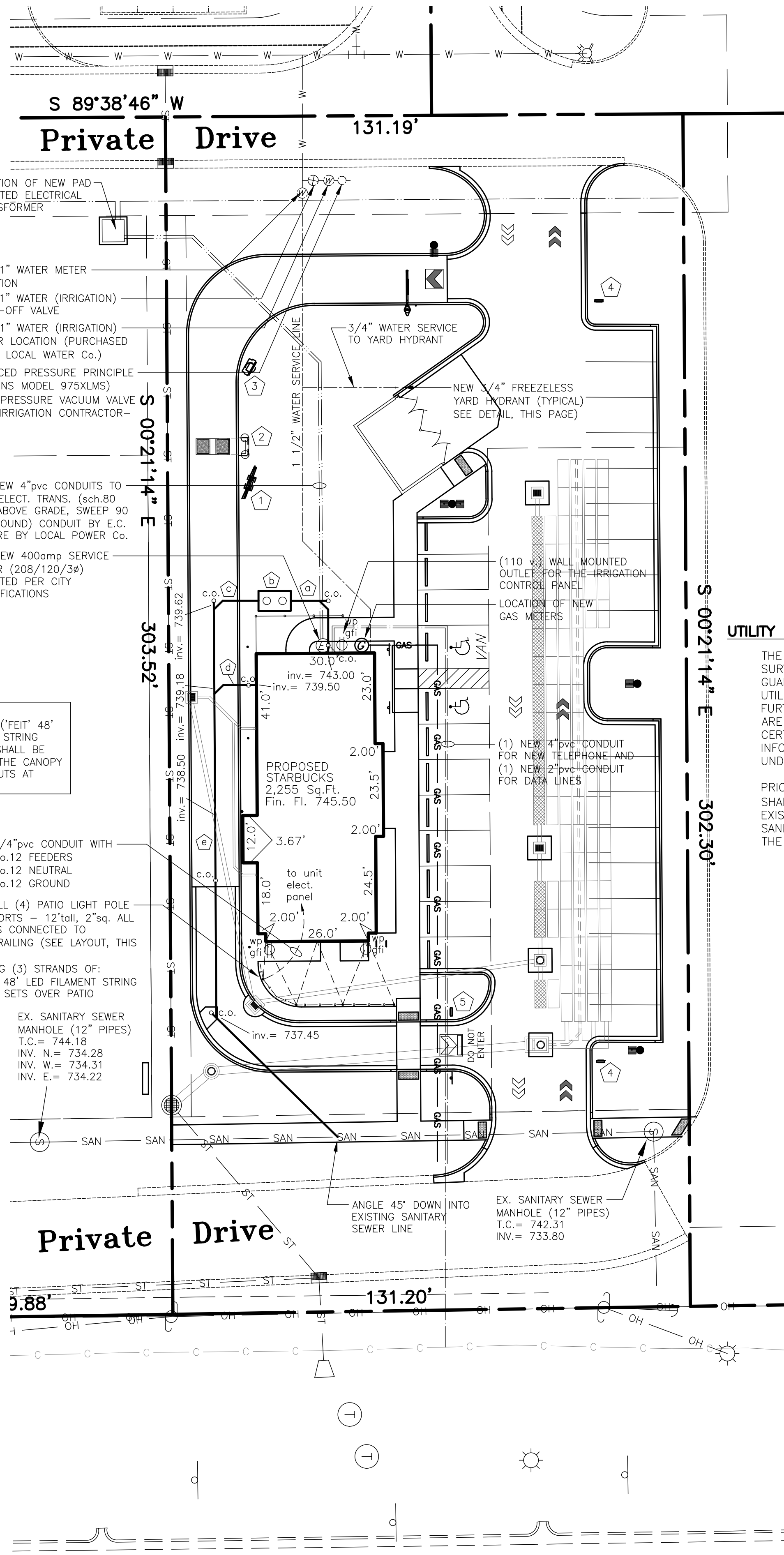
PATIO LIGHT POLE LAYOUT PLAN
SCALE: 1"= 10.0'

INSTALL '1" HOOK 3" FROM TOP OF LIGHT POLES PRIOR TO POWDER COATING. HOOKS TO BE ON PATIO SIDE OF POLE

(1) 3/4"pvc CONDUIT WITH
(1) No.12 FEEDERS
(1) No.12 NEUTRAL
(1) No.12 GROUND
INSTALL (4) PATIO LIGHT POLE SUPPORTS - 12'tall, 2"sq. ALL POSTS CONNECTED TO HANDRAILING (SEE LAYOUT, THIS PAGE)
STRING (3) STRANDS OF: 'FEIT' 48" LED FILAMENT STRING LIGHT SETS OVER PATIO

EX. SANITARY SEWER MANHOLE (12" PIPES)
T.C.= 744.18
INV. N.= 734.28
INV. W.= 734.31
INV. E.= 734.22

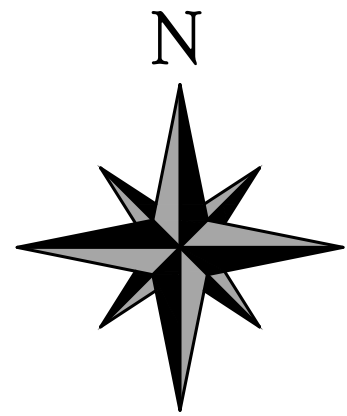
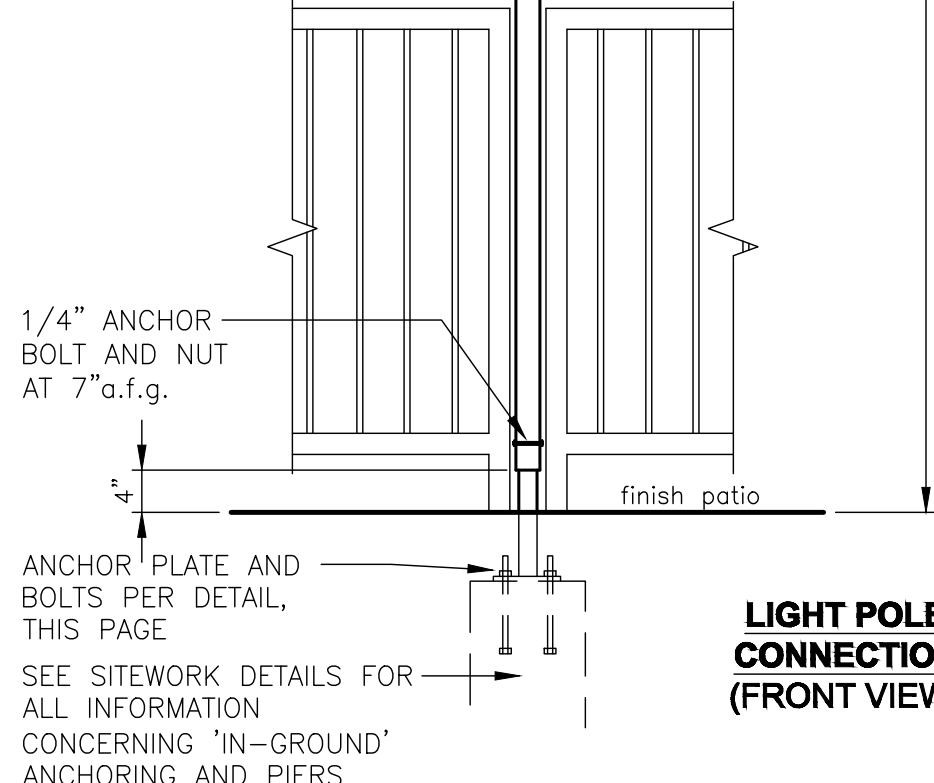
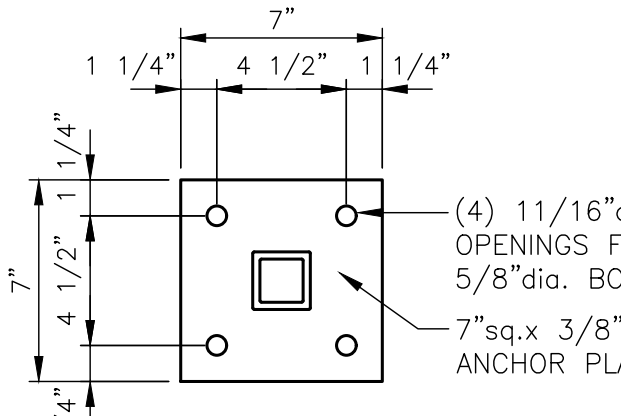
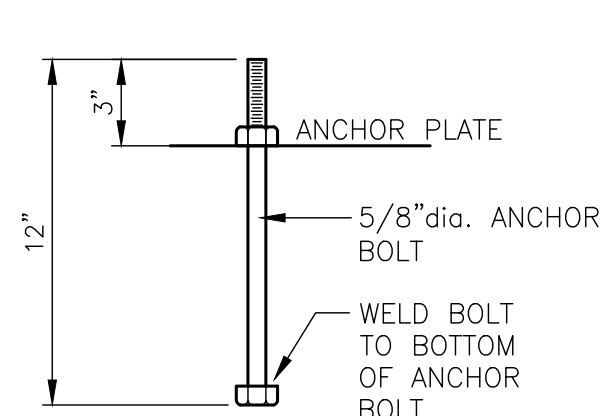
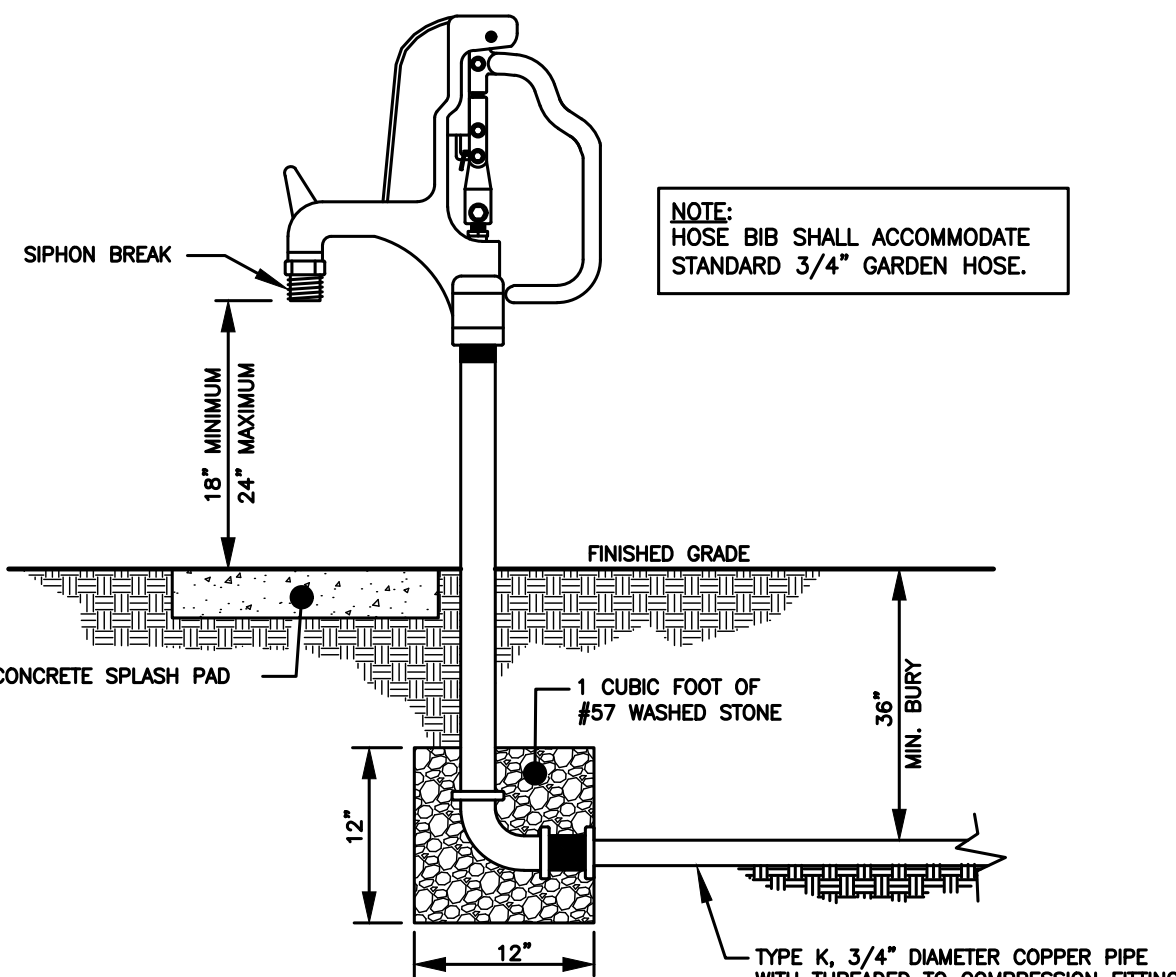
Private Drive



NOTE:
FINAL LOCATION OF NOTES "1 THRU 5" PER TENANT APPROVED PLANS.

CONTRACTOR TO FURNISH AND INSTALL ALL UNDERGROUND ELECTRICAL CONDUITS, LABELED WITH PULL STRINGS FROM THE ELECTRICAL PANELS TO THE FOLLOWING EXTERIOR DRIVE-THRU EQUIPMENT.

- ORDER MENU BOARD:
(1) 1" ELECT. CONDUIT TO ELECT. PANEL WITH LABELED PULL STRING FOR TENANT USE
 - SPEAKER POST/ORDER CONFIRM SYSTEM:
(2) 1" ELECT. CONDUITS FROM DCB (DIGITAL CONTROL BOX)/DOS (DIGITAL ORDER SYSTEM) TO DRIVE-THRU WINDOW AND
(1) 1" CONDUIT FROM DCB (DIGITAL CONTROL BOX)/DOS (DIGITAL ORDER SYSTEM) TO ELECT. PANELS. BOTH WITH LABELED PULL STRING FOR TENANT USE
 - PREVIEW MENU BOARD:
(1) 1" ELECT. CONDUIT TO ELECT. PANEL AND
(1) 1" ELECT. CONDUIT TO ELECT. PANEL FOR FUTURE USE. BOTH WITH LABELED PULL STRING FOR TENANT USE
 - DIRECTIONAL SIGN:
(1) 1" ELECT. CONDUIT TO ELECT. PANEL WITH LABELED PULL STRING FOR TENANT USE
(NOTE: UP TO (3) DIRECTIONAL SIGNS CAN BE SERVED BY A SINGLE CIRCUIT. ADDITIONAL SIGNS REQUIRE A NEW DEDICATED CIRCUIT AND CONDUIT)
 - THANK YOU/DIRECTIONAL SIGN:
(1) 1" ELECT. CONDUIT TO ELECT. PANEL WITH LABELED PULL STRING FOR TENANT USE
- ELECT. CONTRACTOR SHALL INSTALL (1) 1" ELECTRICAL CONDUIT FOR TENANT FURNISHED DETECTOR LOOP TO DCB (DIGITAL CONTROL BOX)/DOS (DIGITAL ORDER SYSTEM) IN COORDINATION WITH TENANT CONSTRUCTION MANAGER.
- ELECT. CONTRACTOR SHALL INSTALL (1) 1" ELECTRICAL CONDUIT TO TENANT FURNISHED DETECTOR LOOP TO DRIVE-THRU WINDOW IN COORDINATION WITH TENANT CONSTRUCTION MANAGER. (LOOP LOCATIONS BY TENANT)
- (1) 1" ELECT. CONDUIT TO BE RUN FROM TENANT'S DEDICATED MONUMENT/PYLON SIGN TO ELECT. PANEL WITH LABELED PULL STRING FOR TENANT USE.
- (1) SPARE 1" ELECT. CONDUIT TO BE RUN THRU FOUNDATION WALL OUT REAR OF BLDG. CAPPED AND TERMINATED ABOVE CEILING. VERIFY LOCATIONS FOR STUB UP WITH TENANT'S REPRESENTATIVE.



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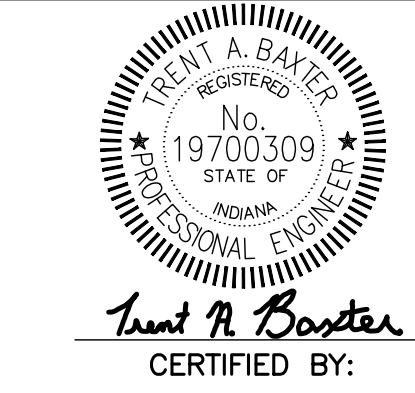
570 East Tracy Road, Suite 610
New Whiteland, Indiana 46184
Ph: 317.535.3579 Fax: 317.535.3581

Starbucks
Franklin Gateway Development
Franklin, Indiana
Site Utility Plan

Job No.: 17023 Date Stamped: 01/21/2019

Drawn By: caw Checked By: tob Scale: 1"= 20.0'

CAD FILE: c:\17023\c300 site utility plan.dwg



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SHEET TITLE:

C300

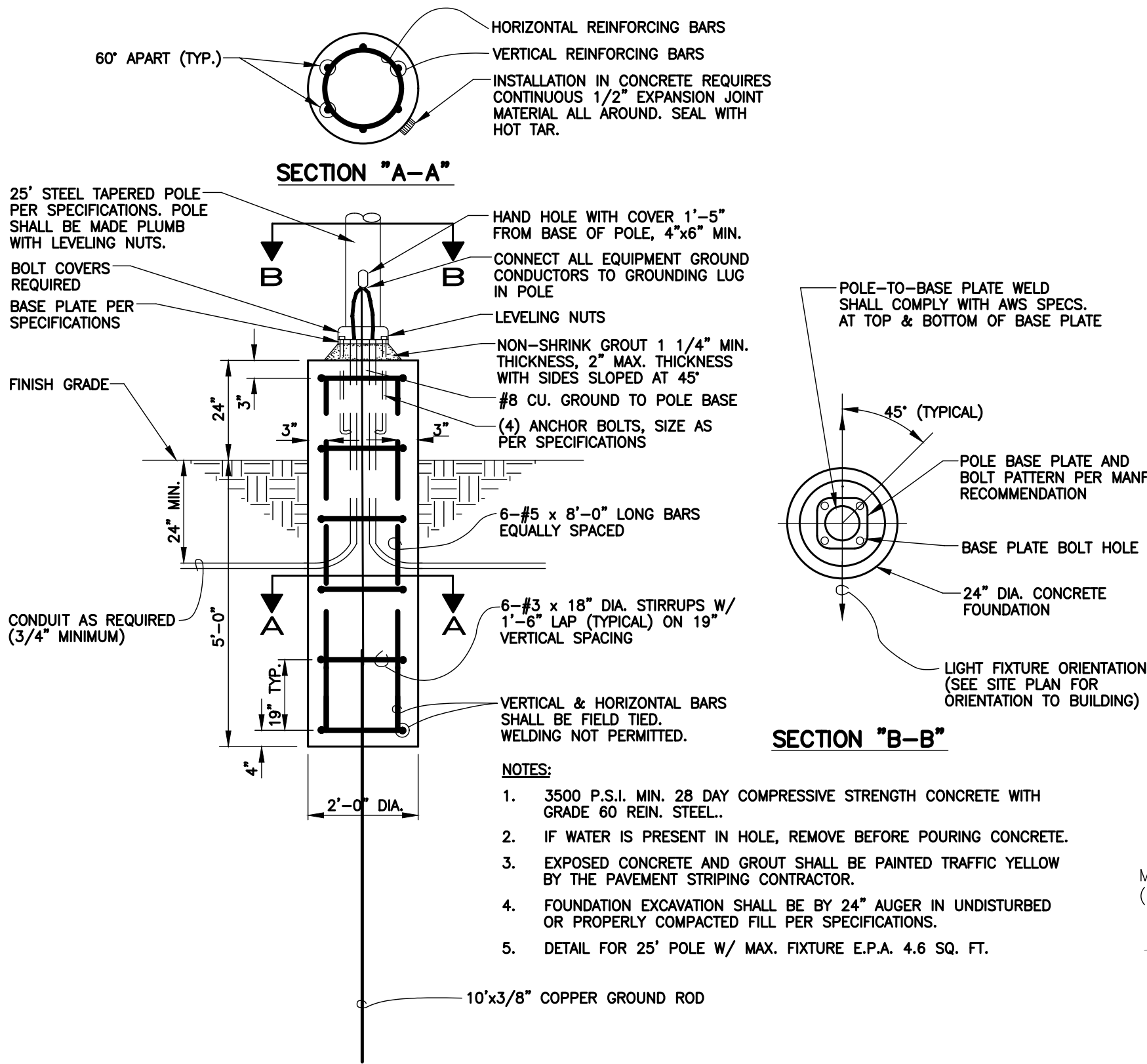


UTILITY DISCLAIMER

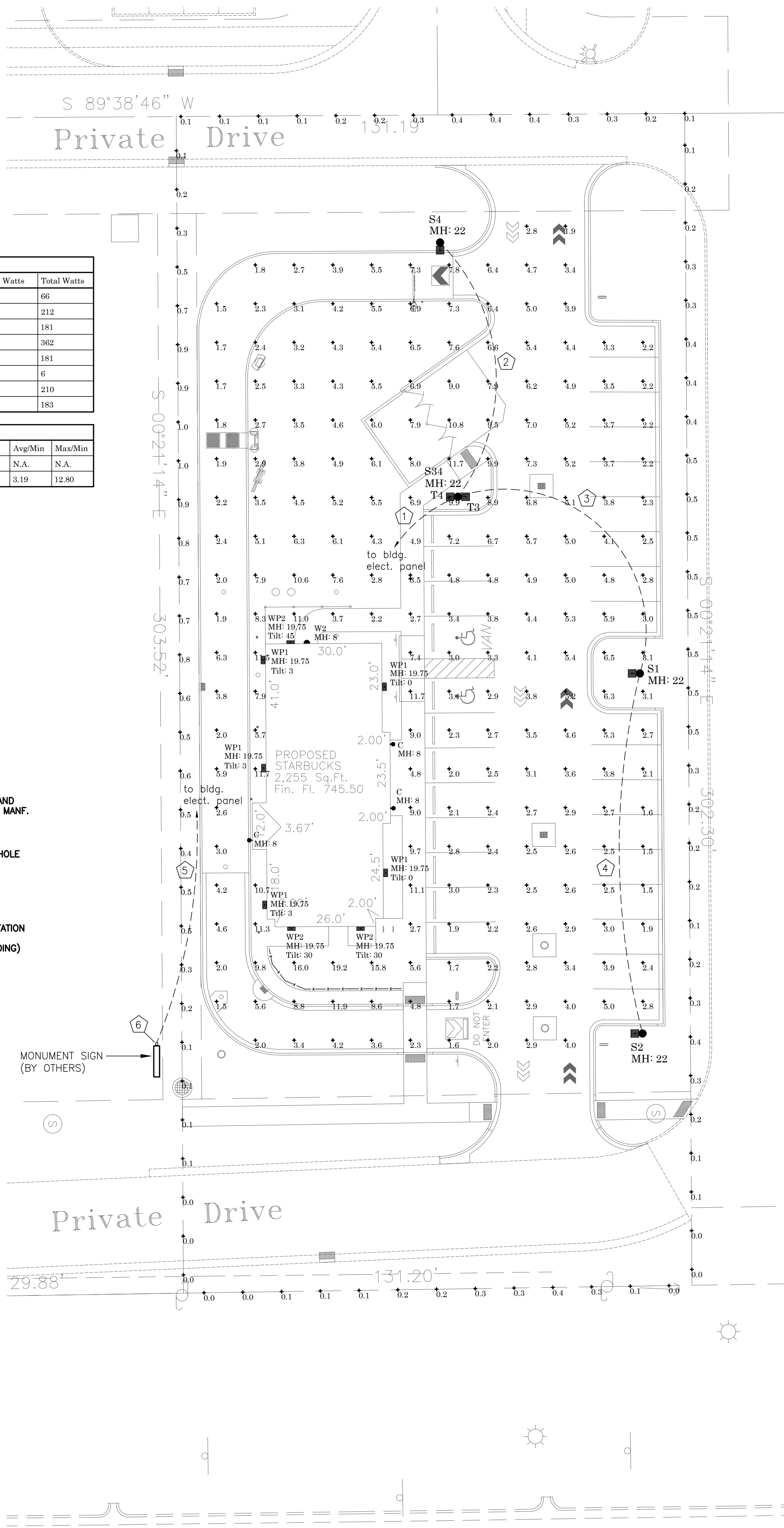
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Luminaire Schedule									
Symbol	Qty	Label	Arrangement	Lum. Lumens	LLF	Description	Lum. Watts	Arr. Watts	Total Watts
	3	C	SINGLE	1660	0.950	LITON WD1362WUE-DUN-GRAY / DOWN CYLINDER / S AFG	22	22	66
	1	S1	SINGLE	17153	0.950	GARCO ECF-S-64L-1A-NW-G2-T3-HIS-MED GRAY / ON 20' POLE ON 2' CONCRETE BASE	212	212	212
	1	S2	SINGLE	15679	0.950	GARCO ECF-S-64L-900-NW-G2-T4-HIS-MED GRAY / ON 20' POLE ON 2' CONCRETE BASE	181	181	181
	1	S34	2@180	19955 / 19112	0.950	(2) GARCO ECF-S-64L-900-NW-G2-T4-MED GRAY + ECF-S-64L-900-NW-G2-T3-MED GRAY / ON 20' POLE ON 2' CONCRETE BASE	181	362	362
	1	S4	SINGLE	19955	0.950	GARCO ECF-S-64L-900-NW-G2-T4-MED GRAY / ON 20' POLE ON 2' CONCRETE BASE	181	181	181
	1	W2	SINGLE	189	0.950	NEW HORIZ NHD2U-GRAY / S AFG	6	6	6
	5	WP1	SINGLE	4948	0.950	LITON WLP31-SILVER-40W-UE-T40-SH / TILTABLE OPTIC / WALL MOUNT WITH SHIELD / 19-9° AFG / TILT PER DRAWING	42	42	210
	3	WP2	SINGLE	7501	0.950	LITON WLP31-SILVER-60W-UE-T40-SH / TILTABLE OPTIC / WALL MOUNT WITH SHIELD / 19-9° AFG / TILT PER DRAWING	61	61	183

Calculation Summary						
Description	Units	Avg	Max	Min	Avg/Min	Max/Min
PROPERTY LINE	Fe	0.35	1.0	0.0	N.A.	N.A.
PARKING AND DRIVE	Fe	4.78	19.2	1.5	3.19	12.80

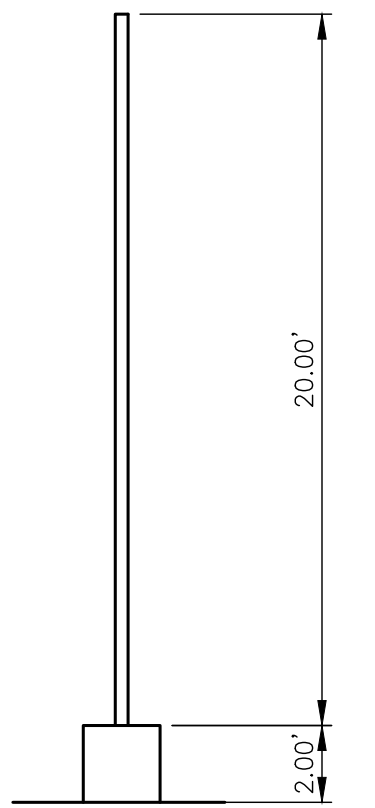


TYPICAL LIGHT POLE FOUNDATION DETAILS
NO SCALE



SITE LIGHTING AND WIRING INFORMATION:

- (1) 3/4" pvc CONDUIT WITH
(2) No.10 FEEDERS
(1) No.10 GROUND
- (1) 3/4" pvc CONDUIT WITH
(2) No.12 FEEDERS
(1) No.12 GROUND
- (1) 3/4" pvc CONDUIT WITH
(2) No.10 FEEDERS
(1) No.10 GROUND
- (1) 3/4" pvc CONDUIT WITH
(2) No.12 FEEDERS
(1) No.12 GROUND
- (1) 3/4" pvc CONDUIT WITH
(2) No.10 FEEDERS
(1) No.10 NEUTRAL
(1) No.10 GROUND
- (1) 6"x 6"x 6" pvc
JUNCTION BOX



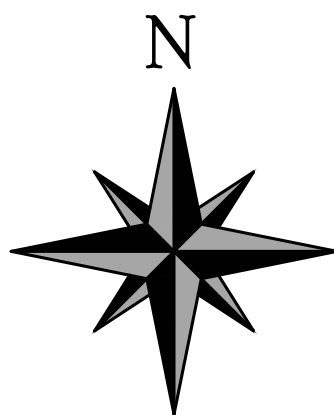
TYPICAL SITE LIGHTING
POLE ELEVATION
NO SCALE

SITE PHOTOMETRIC PLAN
SCALE: 1" = 20.0'



UTILITY DISCLAIMER

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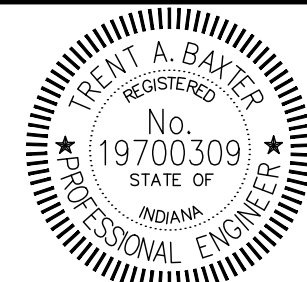
570 East Tracy Road, Suite 610
New Whiteland, Indiana 46154
Ph: 317.535.3579 Fax: 317.535.3581

Starbucks
Franklin Gateway Development
Franklin, Indiana

Site Photometric Plan

Job No.: 17023 Date Stamped: 01/21/2019
Drawn By: caw Checked By: tab Scale: 1" = 20.0'

CAD FILE:
c:\17023\c310 site photometric plan.dwg



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SHEET TITLE:

C310

STORM PIPE NOTES:

- RCP:
RCP shall be Class III, IV, or V in accordance with ASTM C76. A minimum Wall "B" thickness is required. Gasketed joints shall be in conformance with ASTM C443.
- PVC pipe:
(a) PVC solid wall gravity flow storm sewer pipe shall be the bell and spigot type with elastomeric seal joints and smooth inner walls in accordance with ASTM D3034 (4"-15") or ASTM F679 (18"-36"). PVC dual wall gravity flow storm sewer pipe shall be the bell and spigot type with elastomeric seal joints with smooth inner walls and corrugated outer walls in accordance with ASTM F949 (4"-36").
PVC closed profile gravity flow storm sewer pipe shall be the bell and spigot type with elastomeric seal joints with smooth inner and outer walls braced inside circumferentially with projections or ribs in accordance with ASTM F1803 (18"-60").
All PVC pipe shall have a minimum Cell Class as set forth by ASTM D1784 and shall have a minimum pipe stiffness of 46 psi.

(b) Flexible, gasketed joints shall be compression type so that when assembled, the gasket inside the bell is compressed radially on the pipe spigot to form a solight seal. The assembly of joints shall be in accordance with the pipe manufacturer's recommendations, and ASTM D3212. The gasket shall conform to the requirements of ASTM F477. All field cutting of pipe shall be completed in a neat, trim manner using a hand or power saw.

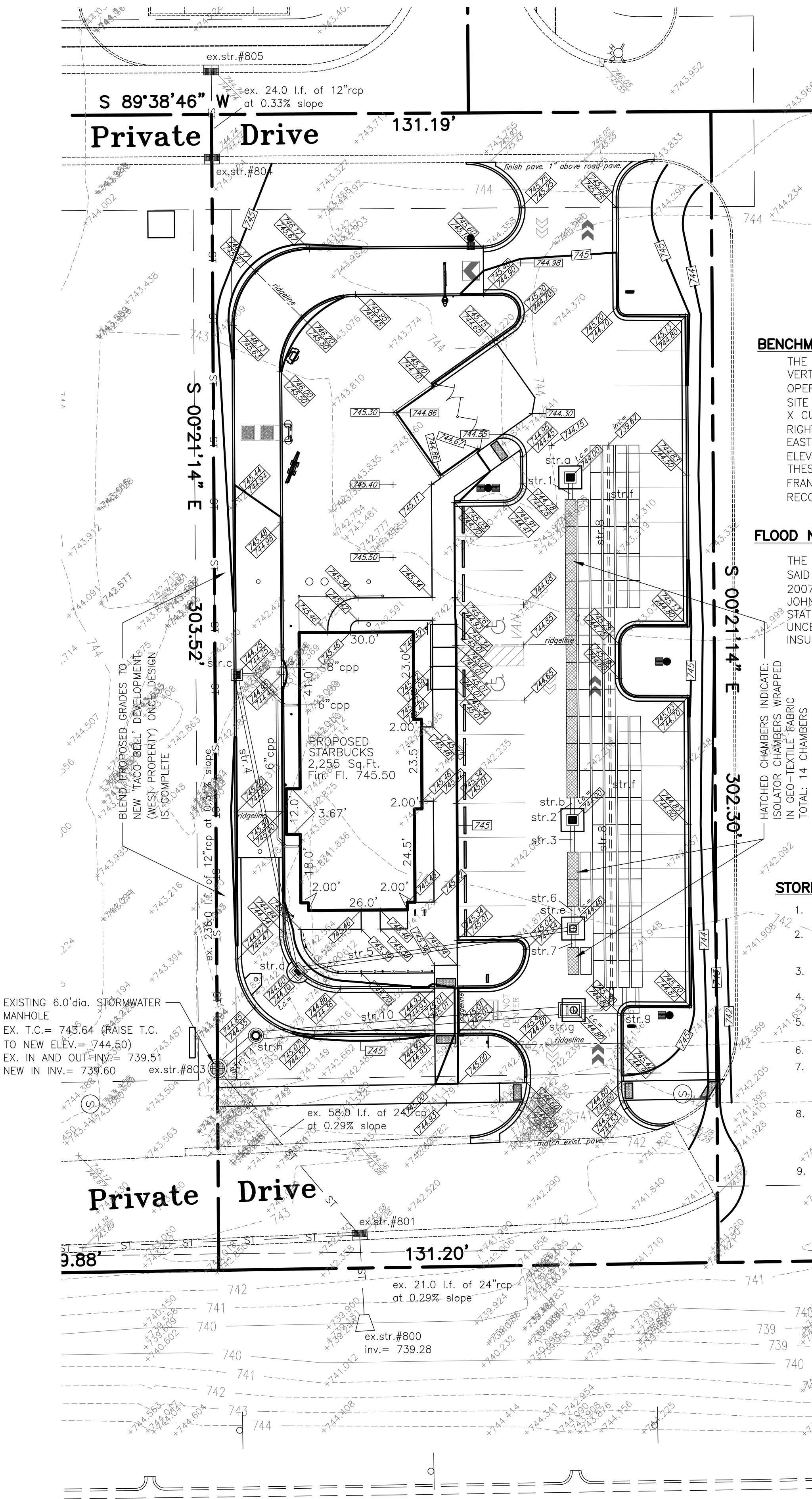
(c) PVC Pipe shall be Type PSM or Pro 21 as manufactured by Diamond Plastics Corporation, Ring-Tite as manufactured by JM Pipe, A-2000 as manufactured by Contech Construction Products, or approved equal.
- RCP shall be provided with Class I or Class II bedding material from a minimum of four (4) inches below the pipe barrel to 1/6 the outside pipe diameter. Within the pavement zone, the backfill shall be Class II material compacted to 95% standard proctor density. Outside the pavement zone, the backfill shall be clean fill material.
- PVC and HDPE pipe shall be provided with Class I bedding material from a minimum of four (4) inches below the pipe barrel to twelve (12) inches above the crown of the pipe. Within the pavement zone, the backfill shall be Class II material compacted to 95% standard proctor density. Outside the pavement zone, the backfill shall be clean fill material.
- Bedding and backfill materials shall be as follows:

a. Class I :
Angular, six (6) to forty (40) millimeters (¼ to 1½ inch) graded stone such as crushed stone. InDOT Classification No. 5, No. 8 and No. 9. A No. 8 washed gravel possessing a minimum 50% mechanical crush count, and meeting the following nominal sizes and percents passing will be considered an equivalent Class I material: 100% passing 1" sieve, 75-95% passing ¾" sieve, 40-70% passing ½" sieve, 20-50% passing ⅜" sieve, 0-15% passing No. 4 sieve, and 0-10% passing No. 8 sieve.

b. Class II:
Coarse sands and gravel-sand mixtures with a maximum particle size of forty (40) millimeters (1½ inches), including variously graded sands and gravels containing small percentages of fine, generally granular and non-cohesive, either wet or dry. Soil types GW, GP, SW and SP are included in this class. InDOT Classification for "B" borrow material.

SITE INFRASTRUCTURE SUMMARY INFORMATION:

INLET/MANHOLE STRUCTURE		PIPE STRUCTURES
● STRUCTURE No.a NEW 24"sq. PRECAST CONCRETE SUMPED STORM INLET WITH GRATED LID CASTING: NEENAH R-3588-A T.C.= 744.00 SOUTH INV. OUT 12"dia. N12 hdpe= 740.77 BOTTOM OF SUMP= 738.77	● STRUCTURE No.g NEW 30"x 5.0' PRECAST CONCRETE STORM MANHOLE WITH SOLID LID CASTING: NEENAH R-1733 T.C.= 744.80 EAST INV. IN 4"pvc (PERFORATED)= 739.67 WEST INV. OUT 24"dia. N12 hdpe= 740.75 TOP OF 36"wide WEIR WALL ELEV.= 741.87 1st- 6"dia. OPENING IN WEIR WALL ELEV.= 740.90 2nd- 2"dia. OPENING IN WEIR WALL ELEV.= 739.67	● NEW STRUCTURE 1 5.8 L.F. OF NEW 12"dia. N12 hdpe AT 0.34% SLOPE INTO ISOLATOR ROW 'STORMTECH' STORAGE CHAMBERS
● STRUCTURE No.b NEW 24"sq. PRECAST CONCRETE SUMPED STORM INLET WITH GRATED LID CASTING: NEENAH R-3588-A T.C.= 744.00 NORTH INV. OUT 12"dia. N12 hdpe= 740.78 SOUTH INV. OUT 12"dia. N12 hdpe= 740.78 BOTTOM OF SUMP= 738.78	● STRUCTURE No.h NEW 30"dia. PRECAST CONCRETE STORM MANHOLE WITH SOLID LID CASTING: NEENAH R-1733 T.C.= 745.00 EAST INV. IN 24"dia. N12 hdpe= 739.61 SOUTH INV. OUT 24"dia. N12 hdpe= 739.61	● NEW STRUCTURE 2 5.8 L.F. OF NEW 12"dia. N12 hdpe AT 0.52% SLOPE INTO ISOLATOR ROW 'STORMTECH' STORAGE CHAMBERS
● STRUCTURE No.c NEW 24"sq. PRECAST CONCRETE STORM INLET WITH CURB INLET LID CASTING: NEENAH R-3236-A T.C.= 744.25 EAST INV. IN 8"cpp= 741.60 SOUTH INV. OUT 12"dia. N12 hdpe= 741.50		● NEW STRUCTURE 3 8.4 L.F. OF NEW 12"dia. N12 hdpe AT 0.34% SLOPE INTO ISOLATOR ROW 'STORMTECH' STORAGE CHAMBERS
● STRUCTURE No.d NEW 30"sq. PRECAST CONCRETE STORM INLET WITH CURB INLET LID CASTING: NEENAH R-3236-A T.C.= 744.10 NORTH INV. IN 12"dia. N12 hdpe= 741.25 EAST INV. OUT 12"dia. N12 hdpe= 741.15		● NEW STRUCTURE 4 79.5 L.F. OF NEW 12"dia. N12 hdpe AT 0.31% SLOPE
● STRUCTURE No.e NEW 24"sq. PRECAST CONCRETE SUMPED STORM INLET WITH SOLID LID CASTING: NEENAH R-1733 T.C.= 744.46 WEST INV. IN 12"dia. N12 hdpe= 740.92 NORTH INV. OUT 12"dia. N12 hdpe= 740.77 SOUTH INV. OUT 12"dia. N12 hdpe= 740.77 BOTTOM OF SUMP= 738.77		● NEW STRUCTURE 5 73.1 L.F. OF NEW 12"dia. N12 hdpe AT 0.31% SLOPE
		● NEW STRUCTURE 6 5.9 L.F. OF NEW 12"dia. N12 hdpe AT 0.34% SLOPE INTO ISOLATOR ROW 'STORMTECH' STORAGE CHAMBERS
		● NEW STRUCTURE 7 5.0 L.F. OF NEW 12"dia. N12 hdpe AT 0.40% SLOPE INTO ISOLATOR ROW 'STORMTECH' STORAGE CHAMBERS
		● NEW STRUCTURE 8 156.5 L.F. OF NEW 4"pvc (PERFORATED) PIPE AT 0.00% SLOPE
● STRUCTURE No.f (100) NEW 'SC-310' STORMTECH STORMWATER STORAGE CHAMBERS ARRANGED PER PLAN NORTH AND SOUTH INV. IN (5) 12"dia. N12 hdpe= 740.75 SOUTH INV. OUT (4) 12"dia. N12 hdpe= 740.75 BOTTOM OF CHAMBERS ELEV.= 740.67 4"pvc PERFORATED UNDERDRAIN/ BOTTOM OF 'BMP' FILTER STONE ELEV.= 739.67		● NEW STRUCTURE 9 24.0 TOTAL L.F. OF NEW 12"dia. N12 hdpe AT 0.00% (MIN.) SLOPE (4 EFFLUENT PIPES FROM STORAGE CHAMBERS) AND 17.1 L.F. OF NEW 24"dia. N12 hdpe (COLLECTION PIPE) AT 0.00% SLOPE
		● NEW STRUCTURE 10 83.8 L.F. OF NEW 24"dia. N12 hdpe AT 0.07% SLOPE
		● NEW STRUCTURE 11 13.3 L.F. OF NEW 24"dia. N12 hdpe AT 0.08% SLOPE



LEGEND:

- EXISTING SPOT ELEVATION
- EXISTING CONTOURS LINES
- EXISTING STORM SEWER LINES
- EXISTING STORM SEWER INLETS
- PROPOSED SPOT ELEVATION
- PROPOSED CONTOURS LINES
- PROPOSED STORM SEWER PIPES
- PROPOSED STORM SEWER INLETS

BENCHMARKS:

THE HORIZONTAL COORDINATE SYSTEM IS NAD 83 INDIANA EAST 1301 AND VERTICAL DATUM IS NAD88 BASED ON THE INDIANA CONTINUOUSLY OPERATING (INCOR).
SITE BENCHMARK (TBM #1)
X CUT ON SE BONNETT BOLT OF FIRE HYDRANT LOCATE ALONG EAST RIGHT OF WAY LINE OF GATEWAY DRIVE. 60' +/- NORTH AND 1' +/- EAST OF SW CORNER OF LOT 2.
ELEV.= 746.69 (NAVD 88).
THESE ELEVATIONS WERE USED BY INDEPENDENT LAND SURVEYING FOR FRANKLIN GATEWAY DEVELOPMENT SECONDARY PLAT- SECTION 2 (NOT RECORDING AT THE TIME OF TOPOGRAPHICAL SURVEY).

FLOOD NOTE:

THE PARCEL DESCRIBED AND SHOWN HEREIN LIES WITHIN ZONE "X" AS SAID PARCEL PLOTS ON MAP NUMBER 18081C0232D (DATED AUGUST 2, 2007) OF THE FLOOD INSURANCE RATE MAPS FOR THE CITY OF FRANKLIN, JOHNSON COUNTY, INDIANA. THE ACCURACY OF THIS FLOOD HAZARD STATEMENT IS SUBJECT TO MAP SCALE UNCERTAINTY AND TO ANY OTHER UNCERTAINTY IN LOCATION OR ELEVATION ON THE REFERENCED FLOOD INSURANCE RATE MAP.

EROSION CONTROL INFORMATION:

SEE SITE EROSION CONTROL PLAN FOR LOCATIONS AND INSTALLATION OF ALL EROSION CONTROL MEASURES REQUIRED ON THIS SITE.

STORM SEWER NOTES:

- ALL STORM SEWERS INCLUDED IN THIS PROJECT SHALL BE PRIVATELY OWNED AND MAINTAINED.
- ALL NEWLY INSTALLED STORM SEWERS SHALL MEET OR EXCEED THE CITY STANDARDS FOR PUBLIC SEWERS AND SHALL BE TESTED PER THE CITY OF FRANKLIN TESTING SPECIFICATIONS.

STORM DRAINAGE AND GRADING NOTES

- ALL NECESSARY PERMITS AND APPROVALS FROM AGENCIES GOVERNING THIS WORK SHALL BE SECURED PRIOR TO BEGINNING CONSTRUCTION.
- CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF ALL PROPERTY CORNERS AND REPLACE ALL PINS ELIMINATED OR DAMAGED DURING CONSTRUCTION.
- ALL PIPES ENTERING STORM SEWER STRUCTURES SHALL BE GROUTED TO ASSURE CONNECTION AT STRUCTURE IS WATER TIGHT.
- DIMENSIONS SHOWN ARE TO CENTERLINE OF PIPE OF CENTERLINE OF STRUCTURE.
- GRADES SHOWN ARE FINISHED GRADES. FOR BUILDING SUBGRADE ELEVATIONS REFER TO ARCHITECTURAL PLANS.
- ALL DIMENSIONS OR COORDINATES SHOWN TO BUILDING ARE TO OUTSIDE
- EXISTING UTILITY LINES SHOWN ARE APPROXIMATE LOCATIONS ONLY. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING UTILITY LINE LOCATIONS PRIOR TO ANY CONSTRUCTION. ANY DEVIATIONS FROM THE DESIGN LOCATIONS SHALL BE REPORTED TO THE OWNER OR ENGINEER
- THE SITE WORK CONTRACTOR SHALL COORDINATE THE INSTALLATION OF ALL UNDERGROUND UTILITIES WITH HIS WORK. ALL UNDERGROUND UTILITIES (WATER, SANITARY SEWER, STORM SEWER, ELECTRICAL CONDUIT, IRRIGATION SLEEVES, AND ANY OTHER MISCELLANEOUS) SHALL BE IN-PLACE PRIOR TO THE PLACEMENT OF BASE COURSE
- ALL FILL AREAS TO BE COMPACTED CLAY ROLLED IN WITH A SHEEPS' FOOT ROLLER IN 8" LIFTS AND COMPACTED TO 95% STANDARD PROCTOR. VERIFICATION OF THE COMPACTION SHALL BE DONE BY AN INDEPENDENT SOILS TESTING COMPANY AND ALL TEST RESULTS SUBMITTED TO THE ENGINEER.

STORM DURATION ELEVATION SUMMARY:

2yr. STORM DURATION:	740.90
10yr. STORM DURATION:	741.35
100yr. STORM DURATION:	741.87

SITE GRADING PLAN
SCALE: 1"= 20.0'



UTILITY DISCLAIMER

EXISTING UNDERGROUND INSTALLATIONS SUCH AS WATER MAINS, GAS MAINS, SEWERS, TELEPHONE LINES, AND BURIED STRUCTURES IN THE VICINITY OF THE WORK TO BE DONE HEREUNDER ARE INDICATED ON THE DRAWINGS ONLY TO THE EXTENT SUCH INFORMATION HAS BEEN MADE AVAILABLE TO OR DISCOVERED BY THE SURVEYOR IN PREPARING THIS DRAWING. THERE IS NO GUARANTEE AS TO THE ACCURACY OR COMPLETENESS OF SUCH INFORMATION, AND ALL RESPONSIBILITY FOR ACCURACY AND COMPLETENESS THEREOF IS EXPRESSLY DISCLAIMED.

REVISION

Starbucks

Franklin Gateway Development

Franklin, Indiana

Site Grading Plan

Job No.: 17023

Date Stamped: 01/21/2019

Drawn By: caw

Checked By: tob

Scale: 1"= 20.0'

CAD FILE: G:\17023\c400 site grading plan.dwg

Trent A. Baxter
CERTIFIED BY:

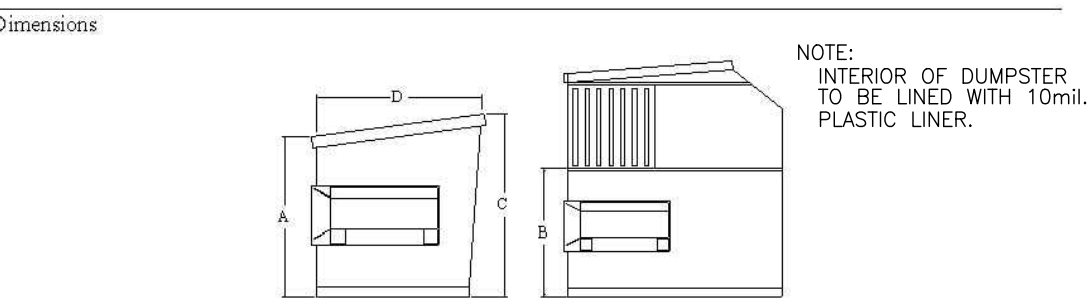
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SHEET TITLE:

C400

Standard Features	
Heavy duty 7 GA. fork sleeves	Debris shedding fork sleeve top gussets
7 GA. fork entry guides	7 GA. fork sleeve backup plates with integral extended bumpers
3" x 4" full length bottom reinforcements	12 GA. side walls
Rugged and light plastic lids	1-1/2" PVC drain plug
Rust resistant primer inside and out	Two coats high gloss enamel finish

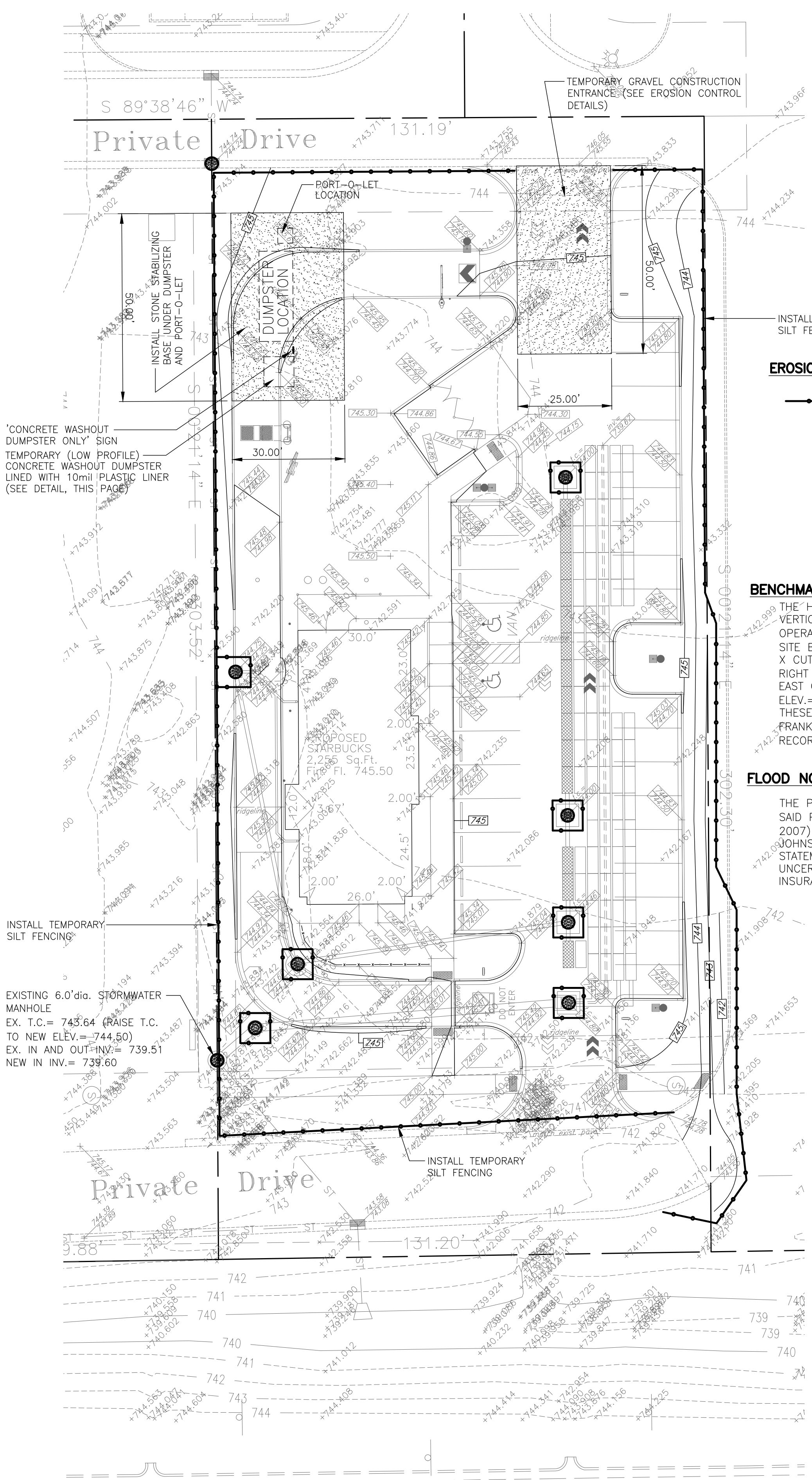
Options	
10 GA. side walls	
7 GA. floor	
Clusters: 2 rigid, 2 swivel, for two through four cubic yard containers	



Model	FL2	FL3	FL4	FL6-LP	FL6	FL6-SL	FL8-LP	FL8	FL8-SL	FL10	FL10-SL
Capacity cu yds	2	3	4	6	6	6	8	8	8	10	10
(A) Loading height	39	48	51	37	63	51	51	72	51	82	51
(B) Side loading height	-	-	-	-	31	-	-	44	-	45	-
(C) Interior height	41	51	60	45	64	60	59	76	72	80	80
(D) Interior depth	36	42	53	96	66	72	96	72	80	75	88
Interior width	72	72	72	72	72	72	72	72	72	72	72
Lid size (2 each)	36x41	36x47	36x58	36x58	36x58	36x58	36x58	36x58	36x58	36x58	36x58
Side door W x H	-	-	-	30x30*	30x30	-	30x30*	30x30	-	30x30	-

All dimensions are in inches
* Available with two side doors or two 36x47 rear lids
-LP = low profile
-SL = slant

CONCRETE WASHOUT DUMPSTER DETAIL



LEGEND:

- EXISTING SPOT ELEVATION
- EXISTING CONTOURS LINES
- EXISTING STORM SEWER LINES
- EXISTING STORM SEWER INLETS
- PROPOSED SPOT ELEVATION
- PROPOSED CONTOURS LINES
- PROPOSED STORM SEWER PIPES
- PROPOSED STORM SEWER INLETS

EROSION CONTROL LEGEND:

- TEMPORARY SILT FENCING (SEE EROSION CONTROL DETAILS PAGE)
- FABRIC DROP INLET EROSION PROTECTION - ONCE PAVEMENT IS PLACED CHANGE TO 'DANDY SACK' PROTECTION (SEE EROSION CONTROL DETAILS PAGE)
- FABRIC DROP INLET EROSION PROTECTION - 'DANDY SACK' (SEE EROSION CONTROL DETAILS PAGE)

BENCHMARKS:
THE HORIZONTAL COORDINATE SYSTEM IS NAD 83 INDIANA EAST 1301 AND VERTICAL DATUM IS NAD88 BASED ON THE INDIANA CONTINUOUSLY OPERATING (INCOR).
SITE BENCHMARK (TBM #1)
X CUT ON SE BONNETT BOLT OF FIRE HYDRANT LOCATE ALONG EAST RIGHT OF WAY LINE OF GATEWAY DRIVE. 60' +/- NORTH AND 1' +/- EAST OF SW CORNER OF LOT 2.
ELEV.= 746.69 (NAVD 88).
THESE ELEVATIONS WERE USED BY INDEPENDENT LAND SURVEYING FOR FRANKLIN GATEWAY DEVELOPMENT SECONDARY PLAT- SECTION 2 (NOT RECORDING AT THE TIME OF TOPOGRAPHICAL SURVEY).

FLOOD NOTE:
THE PARCEL DESCRIBED AND SHOWN HEREIN LIES WITHIN ZONE "X" AS SAID PARCEL PLOTS ON MAP NUMBER 18081C0232D (DATED AUGUST 2, 2007) OF THE FLOOD INSURANCE RATE MAPS FOR THE CITY OF FRANKLIN, JOHNSON COUNTY, INDIANA. THE ACCURACY OF THIS FLOOD HAZARD STATEMENT IS SUBJECT TO MAP SCALE UNCERTAINTY AND TO ANY OTHER UNCERTAINTY IN LOCATION OR ELEVATION ON THE REFERENCED FLOOD INSURANCE RATE MAP.

NOTE:
NO EARTH DISTURBING ACTIVITY MAY COMMENCE WITHOUT AN APPROVED STORM WATER PERMIT.

SEEDING INFO:
1. ALL DISTURBED AREAS SHALL BE PERMINATELY SEEDED PER EROSION CONTROL DETAILS.
(AT NO POINT DURING CONSTRUCTION SHALL ANY PART OF THE SITE REMAIN UNDISTURBED FOR MORE THAN TWO MONTHS THEREFORE NO TEMPORARY SEEDING SHALL BE REQUIRED)

SITE EROSION CONTROL PLAN

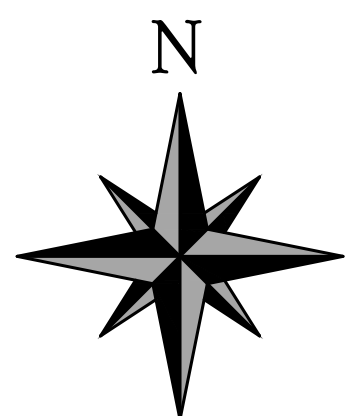
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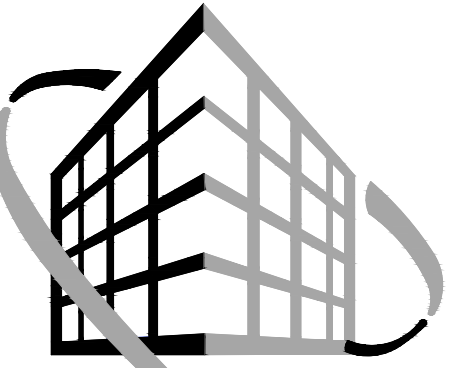
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E. KING ST. (S.R. 44)



REVISION



VERSATILE
CONSTRUCTION
GROUP, LLC.

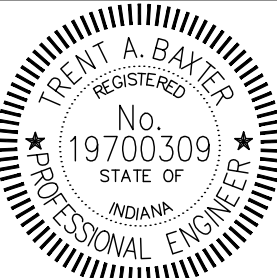
570 East Tracy Road, Suite 610
New Whiteland, Indiana 46184
Ph: 317.535.3579 Fax: 317.535.3581

Starbucks
Franklin Gateway Development
Franklin, Indiana

Site Erosion Control Plan

Job No.:	Date Stamped:
17023	01/21/2019
Drawn By:	Checked By:
caw	tab
Scale:	1"= 20.0'

CAD FILE:
G:\17023\c500 site erosion control plan.dwg



Grant A. Baxter
CERTIFIED BY:

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SHEET TITLE:

C500

EROSION CONTROL RESPONSIBILITY:

INSTALLATION RESPONSIBILITY:

VERSATILE CONSTRUCTION GROUP, LLC
PROJECT SUPERVISOR: MARC TWOREK
570 E. TRACY ROAD, SUITE 610
NEW WHITELAND, INDIANA 46184
PHONE: (317) 535-3579

LAND OWNER:

FRANKLIN RETAIL LLC
6440 WESTFIELD BLVD.
INDIANAPOLIS, INDIANA
PHONE: (317) 472-1800

PROJECT ADDRESS:

STARBUCKS
LOT 3, FRANKLIN GATEWAY DEVELOPMENT, SECTION 2
FRANKLIN, INDIANA 46131

EROSION CONTROL CONTRACTOR RESPONSIBILITY:

RESPONSIBILITIES OF CONTRACTOR REGARDING THE CONSTRUCTION GENERAL PERMIT: The Contractor shall manage the discharge of storm water from the site in accordance with the NPDES Construction General Permit for Construction Activities conditions and the following provisions of this section of the specifications. The Contractor shall be responsible for conducting the storm water management practices in accordance with the permit. The Contractor shall be responsible for providing qualified inspectors to conduct the inspections required by the SWPPP. The Contractor shall be responsible for any enforcement action taken or imposed by federal, state, or local agencies, including the cost of fines, construction delays, and remedial actions resulting from the Contractor's failure to comply with the permit provisions. It shall be the responsibility of the Contractor to make any changes to the SWPPP necessary when the Contractor or any of his subcontractors elects to use borrow or fill or material storage sites, either contiguous to or remote from the construction site, when such sites are used solely for this construction site. Such sites are considered to be part of the construction site covered by the permit and this SWPPP. Off-site borrow, fill, or material storage sites which are used for multiple construction projects are not subject to this requirement, unless specifically required by state or local jurisdictional entity regulations. The Contractor should consider this requirement in negotiating with earthwork subcontractors, since the choice of an off-site borrow, fill, or material storage site may impact their duty to implement, make changes to, and perform inspections required by the SWPPP for the site.

SWPPP LEDGER: Two (2) copies of the SWPPP Ledger, in three (3) ring binders shall be provided by the Operator's Engineer. One (1) copy shall be provided to the Construction Site Superintendent and one (1) copy shall be provided to the Operator's Project Manager.

- Table of Contents
- A copy of the Stormwater Pollution Prevention Plan, Details, and Notes. (Sheets C500, C510, C520, and C530)
- Construction Site Notice (Form A-1)
- Modification Report (Form C-1)
- Reportable Quantity Release Form (Form E-1)
- Construction Site Inspection and Maintenance Log

The Operator's Project Manager must review and evaluate for compliance the SWPPP Ledger at each Project Review.

STORM WATER POLLUTION PREVENTION PROGRAM LOCATION REQUIREMENTS: The SWPPP Ledger is meant to be a working document that shall be maintained at the site of the Construction Activities at all times throughout the project, shall be readily available upon request by the Operator's personnel or IDEM or any other agency with regulatory authority over storm water issues, and shall be kept on-site until the site complies with the Final Stabilization section of this document. A sign or other notice must be posted near the main entrance of the construction site which contains the location of the SWPPP and the name and phone number of a contact person responsible for scheduling SWPPP viewing times, and any other State of Indiana specific requirements. The Notice of Coverage (NOC) or other form notifying the applicant that coverage under the applicable permit has been obtained must also be posted, once received.

SWPPP MODIFICATIONS: The inspection report should also identify if any revisions to the SWPPP are warranted due to unexpected conditions. The SWPPP is meant to be a dynamic working guide that is to be kept current and amended whenever:

- There is a change in design, construction, operation, or maintenance at the construction site that has or could have a significant effect on the discharge of pollutants to the waters of the United States that has not been previously addressed in the SWPPP. In addition to modifying the SWPPP, the site map may also require an amendment.
- Inspections or investigations by site staff, or by local, state or federal officials, determine that the discharges the SWPPP is ineffective in eliminating or significantly minimizing pollutants in storm water discharges from the construction site.
- Based on the results of an inspection, it must be modified as necessary to include additional or modified BMPs designed to correct problems identified. Revisions to the SWPPP must be completed within seven (7) calendar days following the inspection.
- There is a release containing a hazardous substance or oil in an amount equal or in excess of a reportable quantity established under either 40 CFR Part 110, 40 CFR Part 117 or 40 CFR Part 302 occurs during a 24 hour period. Revisions to the SWPPP must be completed within seven (7) calendar days of knowledge of the release.
- There is an off-site borrow or fill area that is used solely for the ATI Physical Therapy construction project. The modification will include, at a minimum, a revision to the SWPPP and site maps and may impact the Contractor's duty to implement and conduct inspections.

Any such changes to the SWPPP must be made in writing on the Requested Changes to the SWPPP (Form C-1) within 7 days of the date such modification or amendment is made. The Contractor's failure to modify the SWPPP to include off-site borrow or fill areas used solely for the project or to monitor or report deficiencies to the Operator will result in the Contractor being liable for fines and construction delays resulting from any federal, state, or local agency enforcement action.

Report to IDEM within 24 hours any noncompliance with the SWPPP that will endanger public health or the environment. Follow up with a written report within 5 days of the noncompliance event. The following events require 24 hour reporting: a) any unanticipated bypass which exceeds any effluent limitation in the permit, b) any upset which exceeds any effluent limitation in the permit, and c) a violation of a maximum daily discharge limitation for any of the pollutants listed by the EPA in the permit to be reported within 24 hours. The written submission must contain a description of the non-compliance and its cause; the period of non-compliance, including exact dates and times, and if the non-compliance has not been corrected, the anticipated time it is expected to continue, and steps taken or planned to reduce, eliminate, and prevent recurrence of the non-compliance.

Record-Keeping Requirements

- The Contractor shall keep the following records related to construction activities at the site:
- Dates when major grading activities occur and the areas which were graded
 - Dates and details concerning the installation of structural controls
 - Dates when construction activities cease in an area
 - Dates when an area is stabilized, either temporarily or permanently
 - Dates of rainfall and the amount of rainfall
 - Dates and descriptions of the character and amount of any spills of hazardous materials
 - Records of reports filed with regulatory agencies if reportable quantities of hazardous materials spilled

CONTROL OF NON-STORM WATER DISCHARGES

Certain types of discharges are allowable under the IDEM General Permit for Construction Activity, and it is the intent of this SWPPP to allow such discharges. These types of discharges will be allowed under the conditions that no pollutants will be allowed to come in contact with the water prior to or after its discharge. The control measures which have been outlined previously in this SWPPP will be strictly followed to ensure that no contamination of these non-storm water discharges takes place. Furthermore, Indiana may prohibit any non-storm water discharges, allow a limited number of types of non-storm water discharges and/or will require coverage for non-storm water discharges under a separate permit. The following non-storm water discharges are allowed by the IDEM and may occur at the job site:

Possible non-storm water discharges that must be diverted to the sanitary system: discharges from fire fighting activities, fire hydrant flushing, potable water sources such as water line flushing, routine exterior building wash-down (with detergents presents). Contractor shall neutralize any super-chlorinated water from water distribution pipes before releasing it into the environment.

Possible non-storm water discharges where detergents are not used should be diverted to the storm sewer system: discharges from waters used to wash vehicles or control dust in order to minimize offsite sediment tracking, pavement wash waters where spills or leaks of hazardous materials have not occurred or detergents have not been used, irrigation drainage from watering vegetation, springs or other uncontaminated groundwater, including dewatering groundwater infiltration.

SPILL RESPONSE FOR CONTRACTOR

Spill Prevention and Response Procedures

The Contractor will train all personnel in the proper handling and cleanup of spilled materials. No spilled hazardous materials or hazardous wastes will be allowed to come in contact with storm water discharges. If such contact occurs, the storm water discharge will be contained on site until appropriate measures in compliance with state and federal regulations are taken to dispose of such contaminated storm water. It shall be the responsibility of the job site superintendent to properly train all personnel in spill prevention and clean up procedures.

- In order to minimize the potential for a spill of hazardous materials to come into contact with storm water, the following steps will be implemented:
 - All materials with hazardous properties (such as pesticides, petroleum products, fertilizers, detergents, construction chemicals, acids, paints, paint solvents, cleaning solvents, additives for soil stabilization, concrete curing compounds and additives, etc.) will be stored in a secure location, with their lids on, preferably under cover, when not in use.
 - The minimum practical quantity of all such materials will be kept on the job site.
 - A spill control and containment kit (containing, for example, absorbent materials, acid neutralizing powder, brooms, dust pans, mops, rags, gloves, goggles, plastic and metal trash containers, etc.) will be provided at the storage site.
 - Manufacturer's recommended methods for spill cleanup will be clearly posted and site personnel will be trained regarding these procedures and the location of the information and cleanup supplies.
- In the event of a spill, the following procedures should be followed
 - All spills will be cleaned up immediately after discovery.
 - The spill area will be kept well ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with the hazardous substances.
 - The project manager and the Engineer of Record will be notified immediately. Spills of toxic or hazardous materials will be reported to the appropriate federal, state, and/or local government agency, regardless of the size of the spill. Spills of amounts that exceed Reportable Quantities of certain substances specifically mentioned in federal regulations (40 CFR 110, 40 CFR 117, and 40 CFR 302) must be immediately reported to the EPA National Response Center, telephone 1-800-424-8802, IDEM, telephone 1-317-233-7745, and the City of Muncie 1-765-747-4845 and the Muncie Fire Department 1-765-747-4870 and the Bureau of Water Quality 1-765-747-4896.
 - If the spill exceeds a Reportable Quantity, the SWPPP must be modified within seven (7) calendar days of knowledge of the discharge to provide a description of the release, the circumstances leading to the release, and the date of the release. The plans must identify measures to prevent the recurrence of such releases and to respond to such releases. Form E-1 must be completed in accordance with this requirement.
- The job site superintendent will be the spill prevention and response coordinator. He will designate the individuals who will receive spill prevention and response training. These individuals will each become responsible for a particular phase of prevention and response. The names of these personnel will be posted in the material storage area and in the

Releases of hazardous substances or oil in excess of reportable quantities (as established under 40 CFR 110, 40 CFR 117, and 40 CFR 302) must be reported. Form E-1 provides further details on the notification and reporting process.

INSPECTIONS AND MAINTENANCE

Inspections and maintenance shall comply with the INSPECTION AND MAINTENANCE DETAILS located on Sheet C530 of these plans. Inspections must be conducted by a "Qualified" Inspector. "Qualified" is defined as a person knowledgeable in the principles and practices of erosion and sediment controls who possesses the skills to assess conditions at the construction site that could impact storm water quality and to assess the effectiveness of any sediment and erosion control measures selected to control the quality of storm water discharges from the construction activity.

INSPECTION AND MAINTENANCE PROCEDURES IN ADDITION TO DETAILS

The following inspection and maintenance practices will be used to maintain erosion and sediment controls and stabilization measures in addition to the INSPECTION AND MAINTENANCE DETAILS:

- All control measures will be inspected as outlined in the INSPECTION GUIDELINES TABLE.
- All measures will be maintained in good working order; if repairs or other measures are found to be necessary, they will be initiated within 24 hours of report.
- Temporary and permanent seeding and all other stabilization measures will be inspected for bare spots, washouts, and healthy growth.
- Gravel shall be added to transit paths as needed to prevent vehicle tracking of sediments.
- The paved streets adjacent to the site entrance will be inspected daily and swept as necessary to remove any excess mud, dirt, or rock tracked from the site.
- The job site superintendent will be responsible for selecting and training the individuals who will be responsible for these inspections, maintenance and repair activities.
- Personnel selected for the inspection and maintenance responsibilities shall be trained in all the inspection and maintenance practices necessary for keeping the erosion and sediment controls that are used onsite in good working order.
- Disturbed areas and materials storage areas will be inspected for evidence of or potential for pollutants entering stormwater systems.

EROSION CONTROL INSTALLATION AND CONSTRUCTION SEQUENCING

The Contractor will be responsible for implementing the following erosion control and storm water management control measures. The Contractor may designate these tasks to certain subcontractors as long as the Contractor retains the responsibility for implementing these controls and ensuring their proper functioning remains with the Contractor. The order of activities will be as follows:

- Conduct an Erosion and Sediment control pre-construction meeting with the City of Franklin and the Storm Water Department.
- Install silt fence perimeter control, tree preservation fencing, and silt inlet protection.
- Install temporary site construction entrance.
- Install dumpster protection including the concrete wash out dumpster.
- Commence site grading and building pad construction.
- Any disturbed areas of the site where construction activity has ceased for more than 14 days shall be temporarily seeded and watered.
- Excavate storm water storage chambers and install all storm sewers and structures.
- Install protection at the locations of all ends of exposed storm sewer pipes.
- Begin construction of buildings.
- Finalize pavement sub-grade preparation.
- Install base material as required for asphaltic pavement.
- Construct all curbs and sidewalks.
- Construct asphaltic pavement.
- Change temporary inlet protection to "Dandy Sacks".
- Carry out final seeding and planting.
- Remove silt fencing and "Dandy Sacks" and rock check dam only after all paving is complete and exposed surfaces are stabilized.

EROSION CONTROL INSTALLATION DETAILS

TREE PRESERVATION

TREE PROTECTION

Protect trees from equipment damage. (Wounds provide entry for insects and disease and reduce transport of sap.)

If trees are damaged, repair immediately. (Repair of wounded areas allows trees to heal quickly, thus reducing insect and disease problems.)

MATERIALS

- Fencing (orange safety fencing for increased visibility), snow fence and support posts.
- Signage.
- Wood mulch, chips, etc.
- Specialized equipment (brush cutter, rotary axe, hand tools).

APPLICATION

- Install fencing around a specimen tree(s) as far out as its crown to keep equipment off the rooting area.
- If a fence cannot be erected, cushion the rooting area with six inches of wood chips, wood, or brick paths.
- Protect trees from equipment damage by creating some type of barrier, fencing them off, or wrapping individual trees with snow fencing.
- Prune low-hanging limbs that could otherwise be broken off by equipment.

TEMPORARY CONSTRUCTION ENTRANCE

DIMENSION

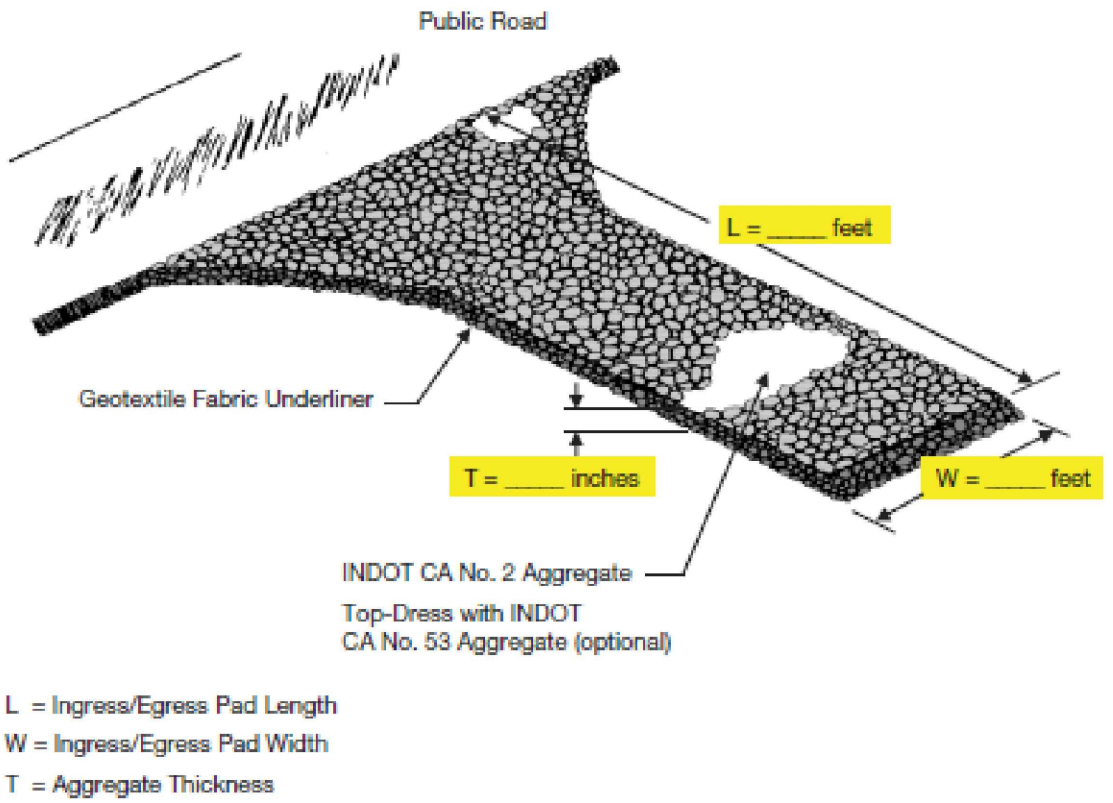
- Width - 12 feet minimum or full width of entrance/exit drive, whichever is greater.
- Length - 50 feet minimum or full length of drive, whichever is greater.
- Thickness - six inches minimum.

MATERIALS

- One to two and one-half inch diameter washed aggregate [INDOT CA No. 2.
- One-half to one and one-half inch washed aggregate [INDOT CA No. 53 optional, used primarily where the purpose of the pad is to keep soil from adhering to vehicle tires].
- Geotextile fabric underlayment (used as a separation layer to prevent intermixing of aggregate and the underlying soil material and to provide greater bearing strength when encountering wet conditions or soils with a seasonal high water table limitation).

INSTALLATION

- Remove all vegetation and other objectionable material from the foundation area.
- Grade the foundation and crown for positive drainage.
- Install a culvert pipe under the pad if needed to maintain proper public road drainage.
- If wet conditions are anticipated, place geotextile fabric on the graded foundation to improve stability.
- Place aggregate (INDOT CA No. 2) to the dimensions and grade shown in the construction plans, leaving the surface smooth and sloped for drainage.
- Top-dress the drive with washed aggregate (INDOT CA No.53).
- Where possible, divert all storm water runoff and drainage from the temporary construction ingress/egress pad to a sediment trap or basin.



L = Ingress/Egress Pad Length
W = Ingress/Egress Pad Width
T = Aggregate Thickness

TOPSOIL SALVAGE AND UTILIZATION

SOIL STOCKPILE AREA

- Free of stumps, rock, and construction debris.
- Stockpile covered with vegetation or a tarp.
- Surrounded by a sediment barrier or sediment filter.

APPLICATION

- Prior to stripping topsoil, install any site-specific down slope measures needed to control storm water runoff and sedimentation.
- Remove soil material no deeper than the surface soil (e.g., A or Ap horizon).
- Stockpiled soil should be temporarily seeded or covered with a tarp and/or surrounded by a sediment control measure.
- Prior to applying topsoil, grade the subsoil and roughen the top three to four inches by disking. (This helps the topsoil bond with the subsoil. If the topsoil and existing soil surface are not properly bonded, water will not infiltrate evenly and it will be difficult to establish vegetation.)
- Apply topsoil evenly to a depth of at least four inches, then compact slightly to improve contact with the subsoil.
 - Depths of four inches or greater are recommended if the underlying material is bedrock, fine-textured clayey soils, loose sand, rock fragments, aggregate, or other unsuitable soil material.
 - Do not apply topsoil when the site is wet, muddy, or frozen because it makes spreading difficult, inhibits bonding, can cause compaction problems, and forms a cloddy seedbed. Whenever possible avoid applying topsoil to the existing soil surface if the two layers have contrasting textures. Clayey topsoil over sandy subsoil is a particularly poor combination, as water creeps along the junction between the two soil layers and may cause the topsoil to slough.
 - Applying topsoil on slopes with a ratio of 2:1 or greater may result in soil slippage and may require additional measures to provide good bonding of the soil material.
- After spreading the topsoil, grade and stabilize the site.

EROSION CONTROL INSTALLATION DETAILS (CONT.)

TEMPORARY SEEDING

MATERIALS

- Soil Amendments - Select materials and rates as determined by a soil test (contact your county soil and water conservation district or cooperative extension office for assistance and soil information, including available soil testing services) or 400 to 600 pounds of 12-0-12 analysis fertilizer, or equivalent. Consider the use of reduced phosphorus application where soil tests indicate adequate phosphorous levels in the soil profile.
- Seed - Select appropriate plant species seed or seed mixtures on the basis of quick germination, growth, and time of year to be seeded (see Table 1).
- Mulch -
 - Straw, hay, wood fiber, etc. (to protect seedbed, retain moisture, and encourage plant growth).
 - Anchored to prevent removal by wind or water or covered with manufactured erosion control blankets.

Table 1. Temporary Seeding Specifications

Seed Species ¹	Rate per Acre	Planting Depth	Optimum Dates ²
Wheat or Rye	150 lbs.	1 to 1½ inches	Sept. 15 – Oct. 30
Spring Oats	100 lbs.	1 inch	March 1 – April 15
Annual Ryegrass	40 lbs.	¼ inch	March 1 – May 1 Aug. 1 – Sept. 1
German Millet	40 lbs.	1 to 2 inches	May 1 – June 1
Sudangrass	35 lbs.	1 to 2 inches	May 1 – July 30
Buckwheat	60 lbs.	1 to 2 inches	April 15 – June 1
Corn (broadcast)	300 lbs.	1 to 2 inches	May 11 – Aug. 10
Sorghum	35 lbs.	1 to 2 inches	May 1 – July 15

¹ Perennial species may be used as a temporary cover, especially if the area to be seeded will remain idle for more than one year (see Permanent Seeding Sheet C510-C520).

² Seeding done outside the optimum seeding dates increases the chances of seeding failure. Dates may be extended or shortened based on the location of the project site within the state.

Notes:

Mulch alone is an acceptable temporary cover and may be used in lieu of temporary seeding, provided that it is appropriately anchored. A high potential for fertilizer, seed, and mulch to wash exists on steep banks, cuts, and in channels and areas of concentrated flow.

SEEDBED PREPARATION

- Test soil to determine pH and nutrient levels.
- Apply soil amendments as recommended by the soil test. If testing is not done, apply 400 to 600 pounds per acre of 12-0-12 analysis fertilizer, or equivalent.
- Work the soil amendments into the upper two to four inches of the soil with a disk or rake operated across the slope.

SEEDING

- Select a seed species or an appropriate seed mixture and application rate from Table 1.
- Apply seed uniformly with a drill or cultipacker seeder or by broadcasting. Plant or cover seed to the depth shown in Table 1.

Notes:

- If drilling or broadcasting the seed, ensure good seed-to-soil contact by firming the seedbed with a roller or cultipacker after completing seeding operations.
- Daily seeding when the soil is moist is usually most effective.
- If seeding is done with a hydroseeder, fertilizer and mulch can be done simultaneously.
- Apply mulch (Mulching Sheet C520) and anchor it in place.

TEMPORARY SEEDING DATES											
	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV
WHEAT OR RYE											
OATS											
ANNU. RYEGRASS											
IRRIGATION NEEDED THIS PERIOD. TO CONTROL EROSION AT TIMES OTHER THAN IN THE SHADED AREA USE MULCH											

PERMANENT SEEDING

MATERIALS

- Soil Amendments - (contact your county soil and water conservation district or cooperative extension office for assistance and soil information, including available soil testing services) or 400 to 600 pounds of 12-0-12 analysis fertilizer, or equivalent. Consider the use of reduced phosphorus application where soil tests indicate adequate phosphorous levels in the soil profile.
- Seed - Select appropriate plant species seed or seed mixtures on the basis of quick germination, growth, and time of year to be seeded (see Table 1).
- Mulch -
 - Straw, hay, wood fiber, etc. (to protect seedbed, retain moisture, and encourage plant growth).
 - Anchored to prevent removal by wind or water or covered with manufactured erosion control blankets.

SITE PREPARATION

- Grade the site to achieve positive drainage.
- Add topsoil (see Topsoil Salvage and Utilization Sheet C510) or mulch (see Mulching Sheet C520) to achieve needed depth for establishment of vegetation. (Compost material may be added to improve soil moisture holding capacity, soil friability, and nutrient availability.)

SEEDBED PREPARATION

- Test soil to determine pH and nutrient levels.
- Apply soil amendments as recommended by the soil test and work into the upper two to four inches of soil. If testing is not done, apply 400 to 600 pounds per acre of 12-0-12 analysis fertilizer, or equivalent.
- Till the soil to obtain a uniform seedbed. Use a disk or rake, operated across the slope, to work the soil amendments into the upper two to four inches of the soil.
- Mulch all seeded areas (see Mulching Sheet C520)

SEEDING

Optimum seeding dates are March 1 to May 10 and August 10 to September 30. Permanent seeding done between May 10 and August 10 may need to be irrigated. Seeding outside or beyond optimum seeding dates is still possible with the understanding that reseeding or overseeding may be required if adequate surface cover is not achieved. Reseeding or overseeding can be easily accomplished if the soil surface remains well protected with mulch.

- Select a seeding mixture and rate from Table 1. Select seed mixture based on site conditions, soil pH, intended land use, and expected level of maintenance.
- Apply seed uniformly with a drill or cultipacker seeder or by broadcasting. Plant or cover the seed to a depth of one-fourth to one-half inch. If drilling or broadcasting the seed, ensure good seed-to-soil contact by firming the seedbed with a roller or cultipacker after completing seeding operations. (If seeding is done with a hydroseeder, fertilizer and mulch can be applied with the seed in a slurry mixture.) and use appropriate methods to anchor the mulch in place. Consider using erosion control blankets on sloping areas and conveyance channels (see Erosion Control Blanket Sheet 520).

REVISION



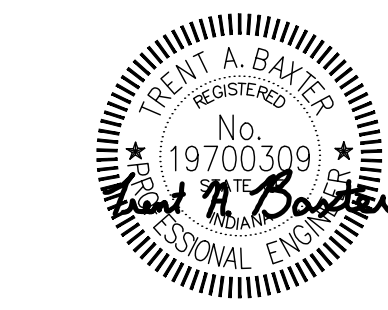
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Starbucks
Franklin Gateway Development
Franklin, Indiana

Erosion Control Details

Job No.:	Date Stamped:	
17023	01/21/2019	
Drawn By:	Checked By:	Scale:
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CAD FILE: G:\17023\c510 erosion control details.dwg		



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SHEET TITLE:

C510

EROSION CONTROL INSTALLATION DETAILS (CONT.)

PERMANENT SEEDING (CONT.)

Table 1. Permanent Seeding Recommendations
This table provides several seed mixture options. Additional seed mixtures are available commercially. When selecting a mixture, consider intended land use and site conditions, including soil properties (e.g., soil pH and drainage), slope aspect, and the tolerance of each species to shade and drought.

Open Low-Maintenance Areas
(remaining idle more than six months)

Seed Mixtures	Rate per Acre Pure Live Seed	Optimum Soil pH
1. Perennial ryegrass - white clover ¹	70 lbs. 2 lbs.	5.6 to 7.0
2. Perennial ryegrass - tall fescue ²	70 lbs. 50 lbs.	5.6 to 7.0
3. Tall fescue ² - white clover ¹	70 lbs. 2 lbs.	5.5 to 7.5

Steep Banks and Cuts, Low-Maintenance Areas (not mowed)

Seed Mixtures	Rate per Acre Pure Live Seed	Optimum Soil pH
1. Smooth brome grass - red clover ¹	35 lbs. 20 lbs.	5.5 to 7.0
2. Tall fescue ² - white clover ¹	50 lbs. 2 lbs.	5.5 to 7.5
3. Tall fescue ² - red clover ¹	50 lbs. 20 lbs.	5.5 to 7.5
4. Orchard grass - red clover ¹ - white clover ¹	30 lbs. 20 lbs. 2 lbs.	5.6 to 7.0
5. Crownvetch ¹ - tall fescue ²	12 lbs. 30 lbs.	5.6 to 7.0

Lawns and High-Maintenance Areas

Seed Mixtures	Rate per Acre Pure Live Seed	Optimum Soil pH
1. Bluegrass	140 lbs.	5.5 to 7.0
2. Perennial ryegrass (turf type)	60 lbs. 90 lbs.	5.6 to 7.0
3. Tall fescue (turf type) ² - bluegrass	170 lbs. 30 lbs.	5.6 to 7.5

Channels and Areas of Concentrated Flow

Seed Mixtures	Rate per Acre Pure Live Seed	Optimum Soil pH
1. Perennial ryegrass - white ¹	150 lbs. 2 lbs.	5.5 to 7.0
2. Kentucky bluegrass - smooth bromegrass - switchgrass - timothy - perennial ryegrass - white clover ²	20 lbs. 10 lbs. 3 lbs. 4 lbs. 10 lbs. 2 lbs.	5.5 to 7.5
3. Tall fescue ¹ - white clover ²	150 lbs. 2 lbs.	5.5 to 7.5
4. Tall fescue ² - perennial ryegrass - Kentucky bluegrass	150 lbs. 20 lbs. 20 lbs.	5.5 to 7.5

Notes:

1. An oat or wheat companion or nurse crop may be used with any of the above permanent seeding mixtures, at the following rates:
(a) spring oats – one-fourth to three-fourths bushel per acre
(b) wheat – no more than one-half bushel per acre
2. A high potential for fertilizer, seed, and mulch to wash exists on steep banks, cuts, and in channels and areas of concentrated flow.

PERMANENT SEEDING DATES											
	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV
Non-irrigated(1)											
Irrigated											
Dormant(2)											
IRRIGATION NEEDED THIS PERIOD. TO CONTROL EROSION AT TIMES OTHER THAN IN THE SHADED AREA USE MULCH											
(1) LATE SUMMER SEEDING DATES MAY BE EXTENDED 5 DAYS IF MULCH IS APPLIED											
(2) INCREASE SEEDING APPLICATION BY 50%											

MULCHING

MATERIALS

Table 1. Mulch Specifications

Material ¹	Rate per Acre	Comments
Straw or hay	2 tons	Should be dry, free of undesirable seeds. Spread by hand or machine. Must be crimped or anchored (see Table 2).
Wood fiber or cellulose	1 ton	Apply with a hydraulic mulch machine and use with tackling agent.

¹ Mulching is not recommended in concentrated flows. Consider erosion control blankets or other stabilization methods.

MULCHING (CONT.)

Table 2. Mulch Anchoring Methods

Anchoring Method ¹	How to Apply
Mulch anchoring tool or farm disk (dull, serrated, and blades set straight)	Crimp or punch the straw or hay two to four inches into the soil. Operate machinery on the contour of the slope.
Cleating with dozer tracks	Operate dozer up and down slope to prevent formation of rills by dozer cleats.
Wood hydromulch fibers	Apply according to manufacturer's recommendations.
Synthetic tackifiers, binders, or soil stabilizers	Apply according to manufacturer's recommendations.
Netting (synthetic or biodegradable material)	Install netting immediately after applying mulch. Anchor netting with staples. Edges of netting strips should overlap with each up-slope strip overlapping four to six inches over the adjacent down-slope strip. Best suited to slope applications. In most instances, installation details are site specific, so manufacturer's recommendations should be followed.

¹ All forms of mulch must be anchored to prevent displacement by wind and/or water.

APPLICATION

1. Apply mulch at the recommended rate shown in Table 1.
2. Spread the mulch material uniformly by hand, hayfork, mulch blower, or hydraulic mulch machine. After spreading, no more than 25 percent of the ground should be visible.
3. Anchor straw or hay mulch immediately after application. The mulch can be anchored using one of the methods listed below:
a. Crimp with a mulch anchoring tool, a weighted farm disk with dull serrated blades set straight, or track cleats of a bulldozer,
b. Apply hydraulic mulch with short cellulose fibers,
c. Apply a liquid tackifier, or
d. Cover with netting secured by staples.

EROSION CONTROL BLANKET

MATERIALS

1. Organic (straw, excelsior, woven paper, coconut fiber, etc.) or synthetic mulch incorporated with a polypropylene, natural fiber or similar netting material (The netting may be biodegradable, photodegradable or permanent.)
2. Six to 12-inch staples, pins, or stakes.

INSTALLATION

1. Select the type and weight of erosion control blanket to fit the site conditions (e.g., slope, channel, flow velocity) per the manufacturer's specifications.
2. Prepare the seedbed, add soil amendments, and permanently seed (see Permanent Seeding Sheet C510, C520) the area immediately following seedbed preparation.
3. Lay erosion control blankets on the seeded area so that they are in continuous contact with the soil with each up-slope or up-stream blanket overlapping the down-slope or down-stream blanket by at least eight inches, or folioe manufacturer's recommendations
4. Tuck the uppermost edge of the upper blankets into a check slot (slit trench), backfill with soil and tamp down. In certain applications, the manufacturer may require additional check slots at specific locations down slope from the uppermost edge of the upper blankets.
5. Anchor the blankets in place by driving staples, pins, or stakes through the blanket and into the underlying soil. Follow an anchoring pattern appropriate for the site conditions and as recommended by the manufacturer.

TURF REINFORCEMENT MAT

MATERIALS

1. Anchoring
Staples, pins, or stakes used to prevent movement or displacement of mat. (Follow manufacturer's recommendations for specific applications.)
2. Turf reinforcement mat (typically consists of a three-dimensional matrix of polypropylene, nylon, or other material).
3. Six to 12-inch staples, pins, or stakes.

INSTALLATION

1. Select a turf reinforcement mat appropriate for the site conditions (e.g., slope, channel, flow velocity) per the manufacturer's specifications.
2. Grade and prepare the soil foundation for mat installation.
3. Install the mat according to the manufacturer's instructions, including burying the edges in check slots or slit trenches.
4. Anchor the mat in place by driving staples, pins, or stakes through the mat and into the underlying soil. Follow an anchoring pattern appropriate for the site conditions and as recommended by the manufacturer.
5. Backfill the mat with topsoil, filling to the top of the mat.
6. Seed the area after the mat has been installed and backfilled with soil.
7. Install erosion control blankets over the seeded turf reinforcement mat to stabilize the surface.

ROCK CHECK DAM

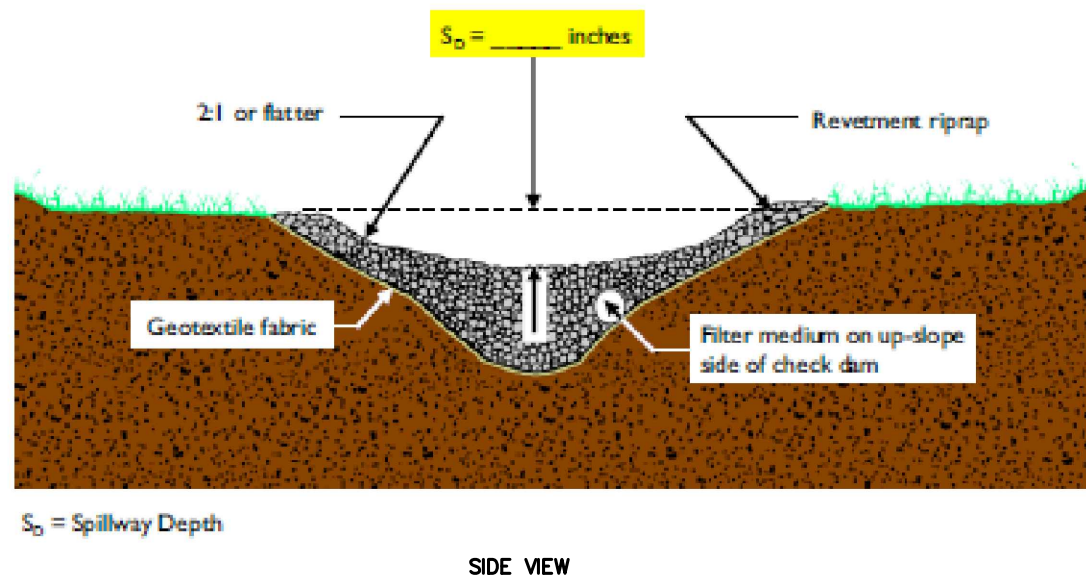
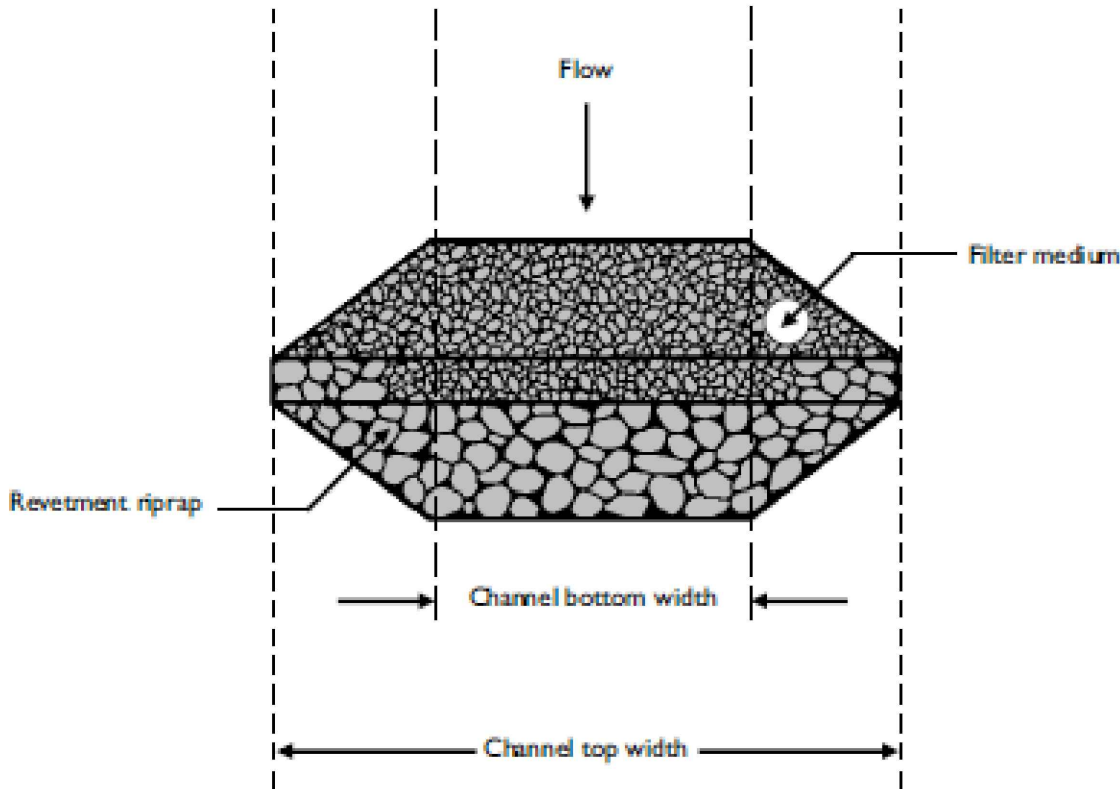
MATERIALS

1. Geotextile fabric (8 ounce or heavier; nonwoven).
2. Indiana Department of Transportation Revetment riprap for dam.
3. INDOT CA No. 5 aggregate for use as filter medium
(Aggregate must be well-graded).

Note: INDOT CA No. 8 aggregate is acceptable if No. 5 aggregate is not available. The use of No. 8 aggregate may result in more frequent overtopping of the structure and will increase the frequency of structure maintenance.

INSTALLATION

1. Lay out the location of the check dam.
2. Excavate a cutoff trench into the channel bottom and ditch banks, extending it a minimum of 18 inches beyond the top of the ditch bank.
3. Install and anchor filter fabric in the channel and cutoff trench.
4. Place riprap in the cutoff trench and channel to the lines and dimensions shown in the construction plans. The center of each dam must be at least nine inches lower than the uppermost points of contact between the riprap dam and channel banks.
5. Extend the riprap at least 18 inches beyond the top of the channel banks to keep overflow water from eroding areas adjacent to the channel banks before it re-enters the channel.
6. Place filter medium (INDOT CA No. 5 aggregate) on the up-slope side of the dam. Place filter medium over the entire face of the dam up to the base of the overflow weir notch.
7. Stabilize the channel above the uppermost dam.
8. Install an erosion-resistant lining in the channel below the lowermost dam. The lining should extend a minimum distance of six feet below the dam.
9. Additional sediment storage can be provided by excavating a small sediment trap on the upstream side of the check dam.



GEOTEXTILE FABRIC DROP INLET PROTECTION

MATERIALS

1. Support posts
2. 2 x 2 inch or 2 x 4 inch hardwood posts.
3. Three feet length, minimum.
4. 1 x 2 inch or 1 x 3 inch hardwood cross bracing lumber.
5. Lathe.
6. Staples or nails.
7. Geotextile fabric

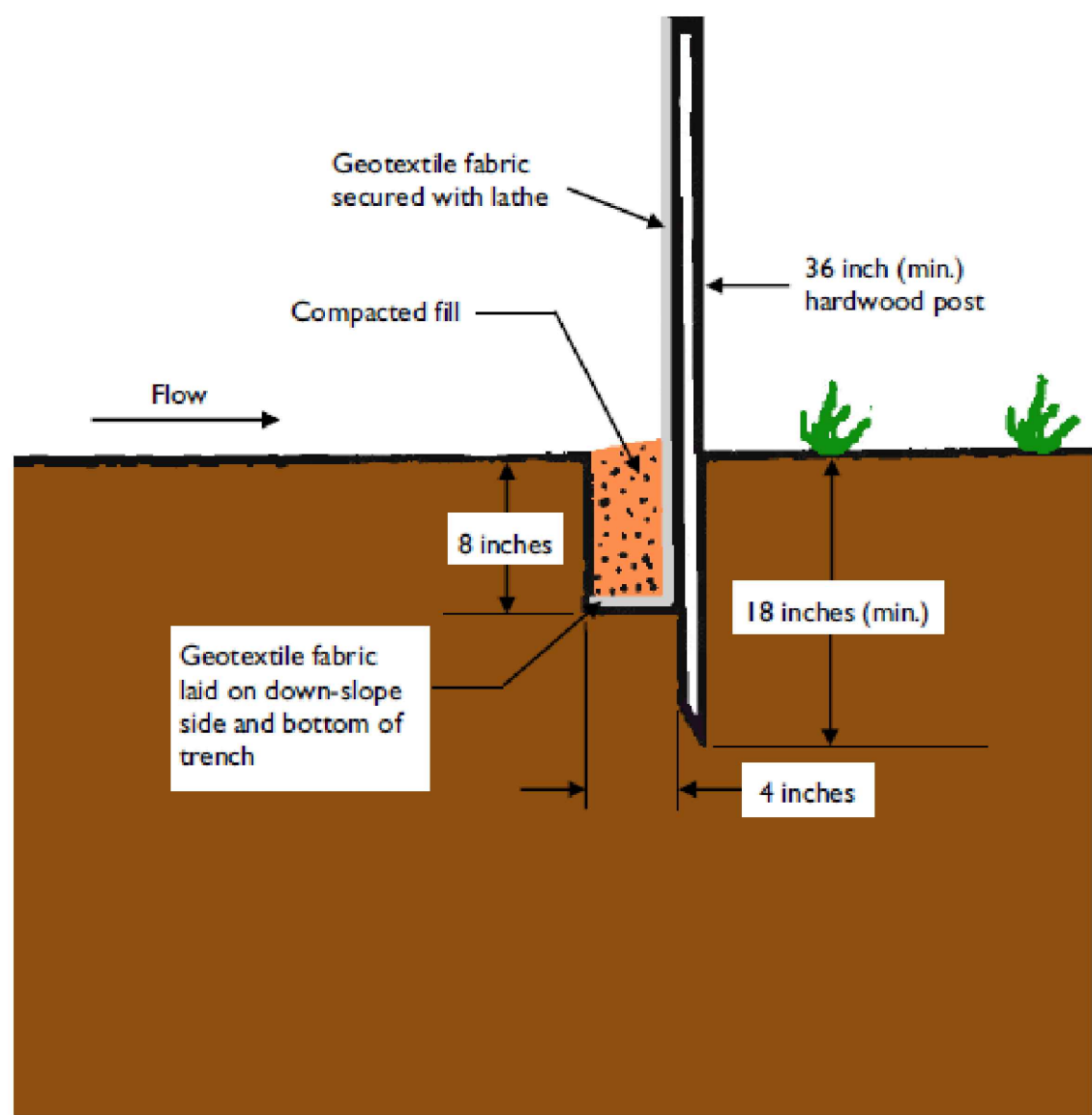
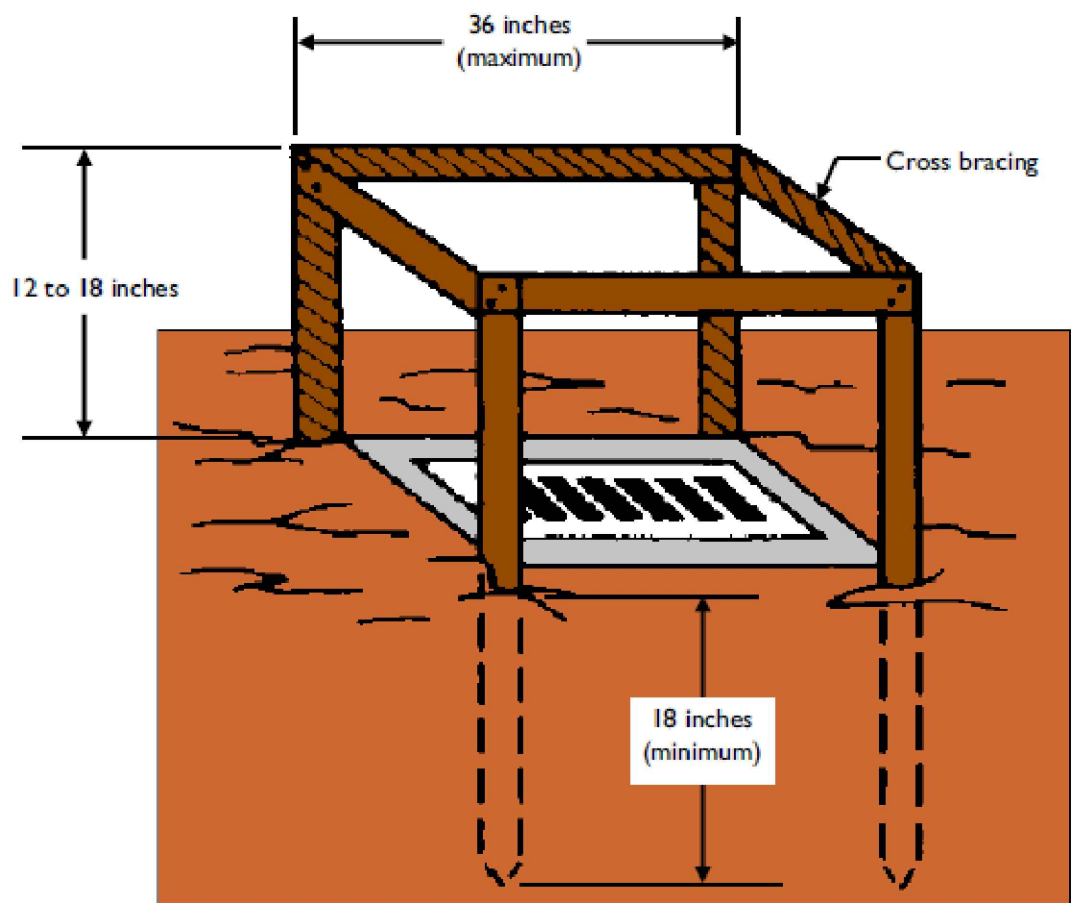
Table 1. Geotextile Fabric Specifications

Physical Property	Woven	Non-Woven
Filtering Efficiency	85%	85%
UV Resistance (Inhibitors and stabilizers to ensure six month minimum life at temperatures of 0° to 120° F)	70%	85%
Tensile Strength at 20% Elongation:		
Standard Strength	30 lbs./linear inch	50 lbs./linear inch
Extra Strength	50 lbs./linear inch	70 lbs./linear inch
Slurry Flow Rate	0.3 gal./min./sq. ft.	4.5 gal./min./sq. ft.
Water Flow Rate	15 gal./min./sq. ft.	220 gal./min./sq. ft.

GEOTEXTILE FABRIC DROP INLET PROTECTION (CONT.)

INSTALLATION

1. Dig an eight-inch deep, four-inch wide trench around the perimeter of the inlet.
2. If using pre-assembled geotextile fabric and posts, drive the posts into the soil, tightly stretching the geotextile fabric between posts as each is driven. (Posts must be placed on the inlet side of the anchor trench with the geotextile fabric on the side of the trench farthest from the inlet.)
Note: If assembling the geotextile fabric and posts on-site, drive the posts into the soil and then secure the geotextile fabric to the posts by placing a piece of lathe over the fabric and fastening it to the post (stretching the fabric between posts as it is fastened).
3. Use the wrap join method when joining posts (see Silt Fence on sheet C520, C530).
4. Place the bottom 12 inches of geotextile fabric into the eight-inch deep trench, laying the remaining four inches in the bottom of the trench and extending away from the inlet.
5. Backfill the trench with soil material and compact it in place.
6. Brace the posts by nailing braces into each corner post or utilize rigid panels to support fabric.
Note: In situations where storm water may bypass the structure, either:
1. Set the top of the geotextile fabric filter at least six inches lower than the ground elevation on the down-slope side of the storm drain inlet,
2. Build a temporary dike, compacted to six inches higher than the fabric, on the down-slope side of the storm drain inlet, AND/OR



SILT FENCE

MATERIALS

1. Fabric – woven or non-woven geotextile fabric meeting specified minimums outlined in Table 2.
a. Height – a minimum of 18 inches above ground level (30 inches maximum).
b. Reinforcement – fabric securely fastened to posts with wood lathe.
2. Support Posts
a. 2 x 2 inch hardwood posts. Steel fence posts may be substituted for hardwood posts (steel posts should have projections for fastening fabric).
b. Spacing – Eight feet maximum if fence is supported by wire mesh fencing. Six feet maximum for extra-strength fabric without wire backing.

REVISION



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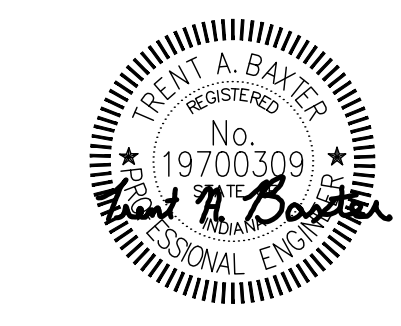
Starbucks
Franklin Gateway Development
Franklin, Indiana

Erosion Control Details

Job No.: 17023 Date Stamp: 01/21/2019

Drawn By: tab Checked By: tab Scale:

CAD FILE:
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C520

EROSION CONTROL INSTALLATION DETAILS (CONT.)

SILT FENCE (CONT.)

Table 1. Slope Steepness Restrictions

Percent Slope		Maximum Distance
< 2%	< 50:1	100 feet
2% – 5%	50:1 to 20:1	75 feet
5% – 10% ¹	20:1 to 10:1	50 feet
10% – 20% ¹	10:1 to 5:1	25 feet
> 20% ¹	> 5:1	15 feet

¹ Consider other alternatives.

Note: Multiple rows of silt fence are not recommended on the same slope.

Table 2. Geotextile Fabric Specifications for Silt Fence (minimum)

Physical Property	Woven Geotextile Fabric	Non-Woven Geotextile Fabric
Filtering efficiency	85%	85%
Textile strength at 20% elongation Standard strength Extra strength	30 lbs. per linear inch 50 lbs. per linear inch	50 lbs. per linear inch 70 lbs. per linear inch
Slurry flow rate	0.3 gal./min./square feet	4.5 gal./min./square feet
Water flow rate	15 gal./min./square feet	220 gal./min./square feet
UV resistance	70%	85%
Post spacing	7 feet	5 feet

Note: Silt fences can be purchased commercially.

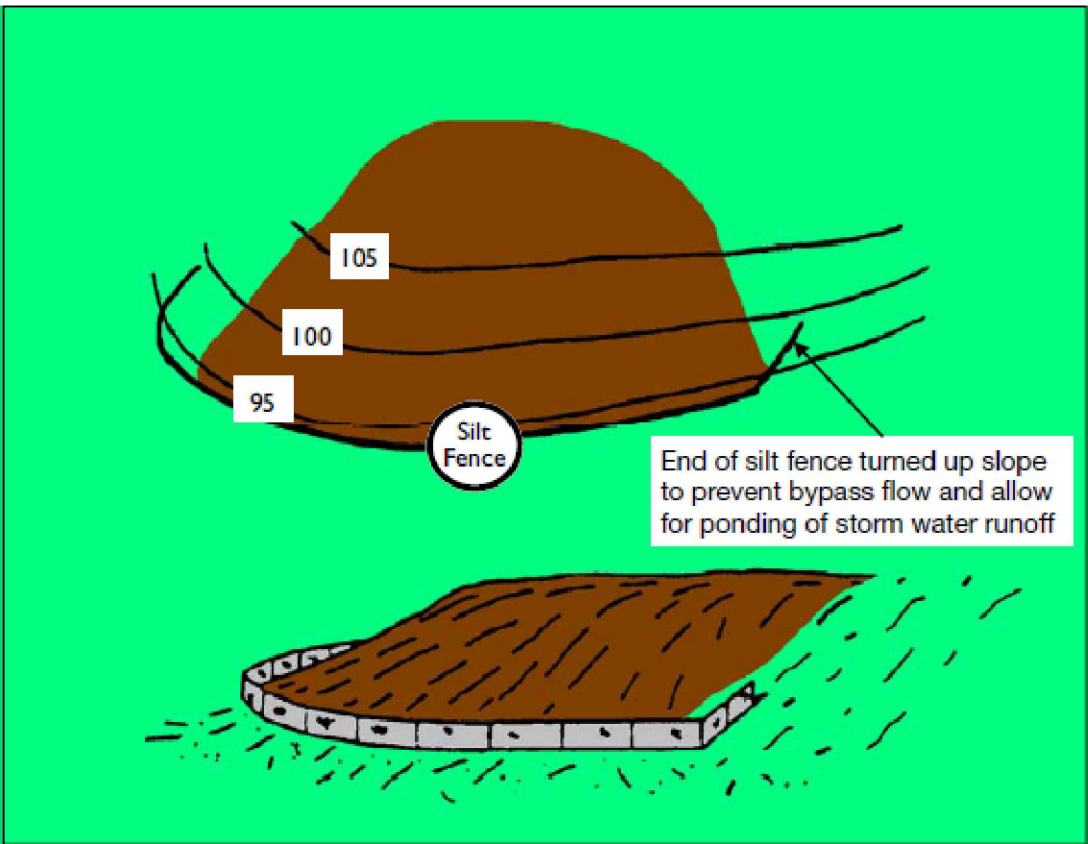
INSTALLATION

Prefabricated silt fence (see Exhibits 1, 2, and 3)

- Lay out the location of the fence so that it is parallel to the contour of the slope and at least 10 feet beyond the toe of the slope to provide a sediment storage area. Turn the ends of the fence up slope such that the point of contact between the ground and the bottom of the fence end terminates at a higher elevation than the top of the fence at its lowest point (see Exhibit 1).
- Excavate an eight–inch deep by four–inch wide trench along the entire length of the fence line (see Exhibit 2). Installation by plying is also acceptable.
- Install the silt fence with the filter fabric located on the up–slope side of the excavated trench and the support posts on the down–slope side of the trench.
- Drive the support posts at least 18 inches into the ground, tightly stretching the fabric between the posts as each is driven into the soil. A minimum of 12 inches of the filter fabric should extend into the trench. (If it is necessary to join the ends of two fences, use the wrap joint method shown in Exhibit 3.)
- Lay the lower four inches of filter fabric on the bottom of the trench and extend it toward the up–slope side of the trench.
- Backfill the trench with soil material and compact it in place.

Note: If the silt fence is being constructed on–site, attach the filter fabric to the support posts (refer to Tables 1 and 2 for spacing and geotextile specifications) and attach wooden lathe to secure the fabric to the posts. Allow for at least 12 inches of fabric below ground level. Complete the silt fence installation, following steps 1 through 6 above.

Exhibit 1



SILT FENCE (CONT.)

Exhibit 2

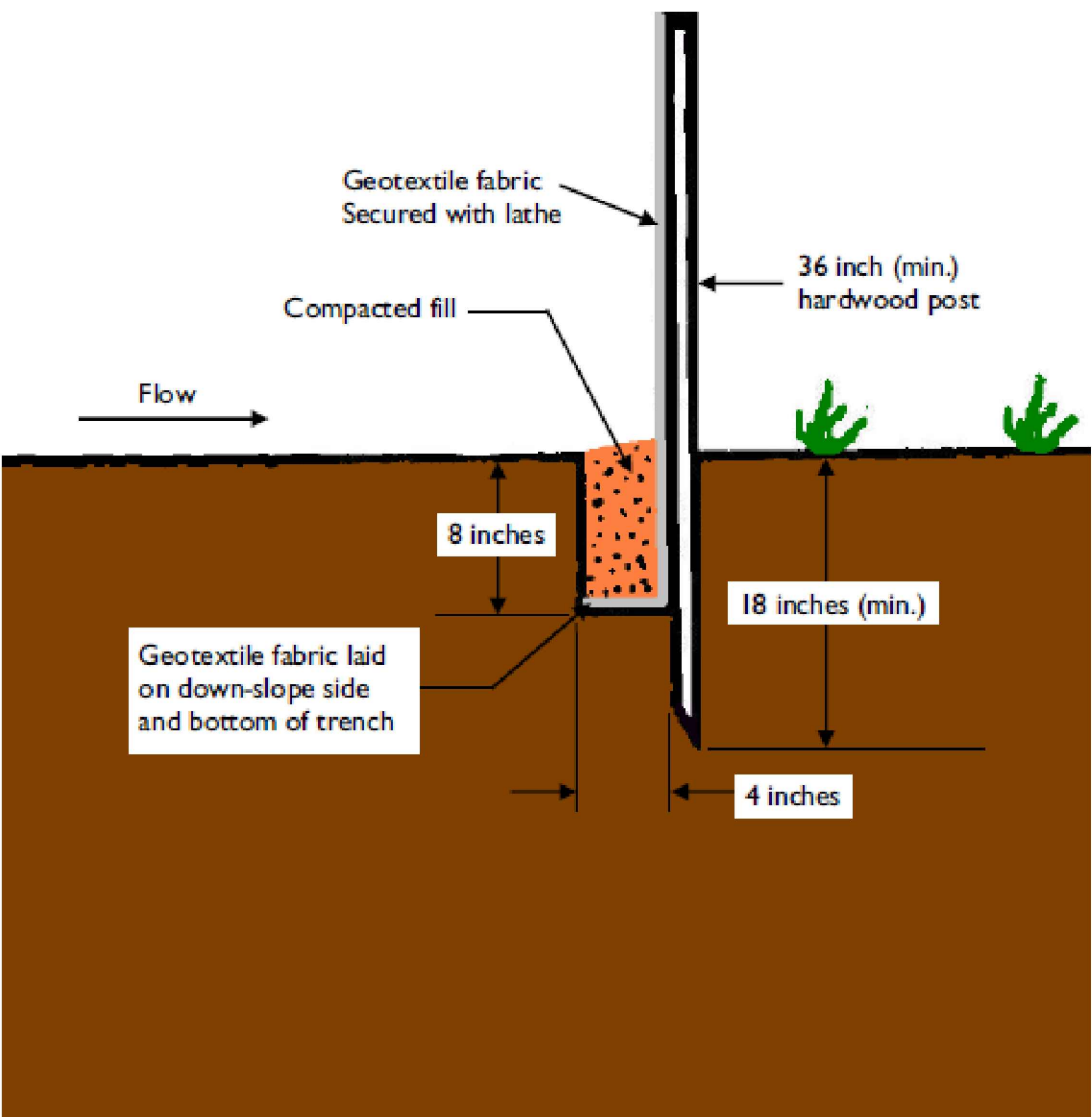
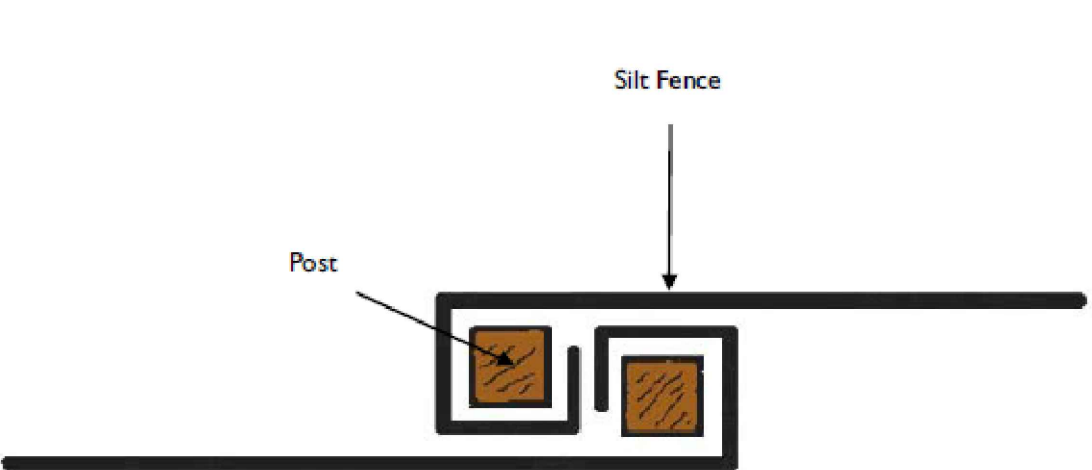


Exhibit 3



PREFABRICATED CONCRETE WASHOUT SYSTEM/CONTAINER MATERIALS

- Self–contained sturdy containment systems that are delivered to a site
- Minimum of ten millimeter polyethylene sheeting that is free of holes, tears, and other defects. The sheeting selected should be of an appropriate size to fit the washout system without seams or overlap of the lining
- Signage.

SURFACE ROUGHENING

Roughening Slopes (To Be Mowed)

- Make slopes to be mowed no steeper than 3:1.
- Use a tiller, disk, harrow, or culti–packer to roughen the slopes, creating shallow grooves no more than ten inches apart, one inch deep, and that run parallel to the slope contour.

Roughening Areas with Tracked Machinery

- Limit roughening with tracked machinery to sandy or relatively dry, finetextured soils to avoid undue surface compaction. (This roughening method is generally not as effective as other roughening methods).
- Operate the tracked machinery up and down the slope so as to leave horizontal depressions in the soil.

Note: Do not back–blade during the final grading operation.

Seeding, Fertilizing, and Mulching Roughened Areas

- Immediately seed, fertilize, and mulch surface–roughened areas while soil is loose and moist to aid seed germination and vegetative growth (see Temporary Seeding Sheet C510; Permanent Seeding Sheet C510, C520; Mulching Sheet C520)
- If roughening with tracked machinery, consider seeding, fertilizing, and mulching first, letting the cleats of the tracks incorporate the seed and fertilizer into the soil and anchor the mulch. This is especially well suited for temporary seeding when timeliness is critical and equipment is unavailable for planting operations.

DANDY SACKS (PAVED INLET DROP PROTECTION)

MATERIALS

THE DANDY SACK™ WILL BE MANUFACTURED IN THE U.S.A. FROM A WOVEN MONOLAMINANT FABRIC THAT MEETS OR EXCEEDS THE FOLLOWING SPECIFICATIONS:

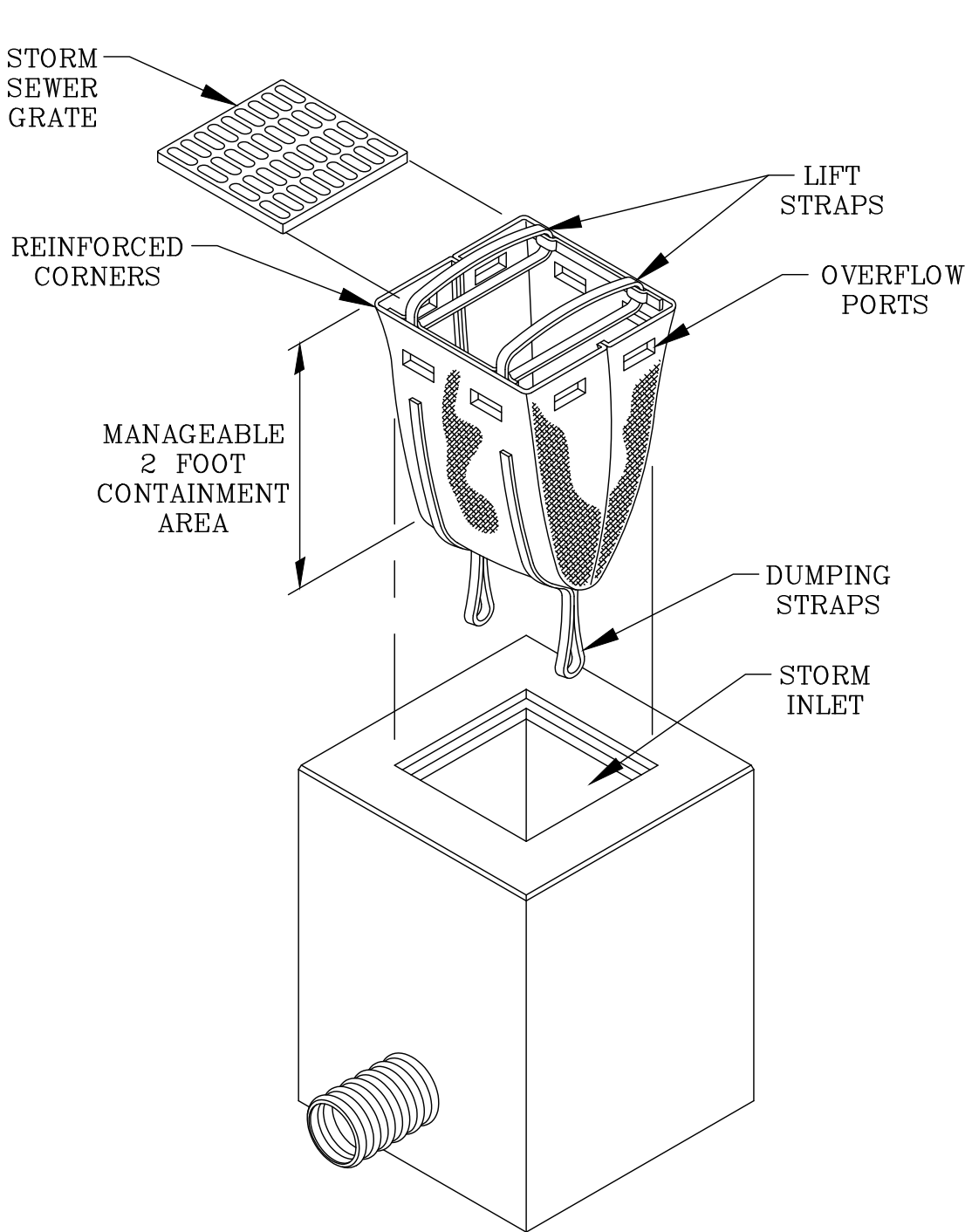
REGULAR FLOW DANDY SACK™ (BLACK)

Mechanical Properties	Test Method	Units	MARV
Grab Tensile Strength	ASTM D 4632	kN (lbs)	1.78 (400) x 1.40 (315)
Grab Tensile Elongation	ASTM D 4632	%	15 x 15
Puncture Strength	ASTM D 4833	kN (lbs)	0.67 (150)
Mullen Burst Strength	ASTM D 3786	kPa (psi)	9506 (800)
Trapezoid Tear Strength	ASTM D 4433	kN (lbs)	0.67 (150) x 0.73 (165)
UV Resistance	ASTM D 4355	%	90
Apparent Opening Size	ASTM D 4751	Mm (US Std. Sieve)	0.425 (40)
Flow Rate	ASTM D 4491	l/min/m ² (gal/min/ft ²)	2852 (70)
Permittivity	ASTM D 4491	Sec ⁻¹	0.90

HI-FLOW DANDY SACK™ (SAFETY ORANGE)

Mechanical Properties	Test Method	Units	MARV
Grab Tensile Strength	ASTM D 4632	kN (lbs)	1.62 (365) x 0.89 (200)
Grab Tensile Elongation	ASTM D 4632	%	24 x 10
Puncture Strength	ASTM D 4833	kN (lbs)	0.40 (90)
Mullen Burst Strength	ASTM D 3786	kPa (psi)	3097 (450)
Trapezoid Tear Strength	ASTM D 4433	kN (lbs)	0.51 (115) x 0.33 (75)
UV Resistance	ASTM D 4355	%	90
Apparent Opening Size	ASTM D 4751	Mm (US Std. Sieve)	0.425 (40)
Flow Rate	ASTM D 4491	l/min/m ² (gal/min/ft ²)	5907 (145)
Permittivity	ASTM D 4491	Sec ⁻¹	2.1

DANDY SACKS (PAVED INLET DROP PROTECTION) (CONT.)



EROSION CONTROL INSPECTION AND MAINTENANCE GUIDELINES

EROSION CONTROL INSPECTION GUIDELINES TABLE

EROSION CONTROL MEASURE	EROSION CONTROL MEASURE INSPECTION FREQUENCY			
	DAILY	WEEKLY	AFTER RAIN EVENT	AS NEEDED
TREE PRESERVATION		X		X
CONSTRUCTION ENTRANCE	X			X
TOP SOIL STOCKPILE	X			X
TEMPORARY SEEDING		X	X	X
PERMANENT SEEDING		X	X	X
MULCHING		X	X	X
CONTROL BLANKET		X	X	X
TURF REINFOR. MAT		X	X	X
ROCK CHECK DAM		X	X	X
FABRIC INLET	X			X
SILT FENCE		X	X	X
CONCRETE WASHOUT	X		X	X
SURFACE ROUGHENING			X	X
DANDY SACKS		X	X	X
DUST CONTROL				X

EROSION CONTROL MAINTENANCE GUIDELINES

TREE PRESERVATION

- Inspect at least once every seven calendar days.
- Repair perimeter barriers if damaged.
- Inspect for damage from construction equipment, etc. Repair wounds simply by removing damaged bark and wood tissue. Do not use tree paint.
- Cable and brace any trunk splits, weak forks, and large limbs.
- Properly prune all damaged limbs. Avoid leaving stubs.
- Aerate soil where compaction has been excessive.
- Fertilize to improve tree growth, vigor, and appearance.
- Water during dry periods to help offset soil compaction and root damage.

TEMPORARAY CONSTRUCTION ENTRANCE

- Inspect daily.
- Reshape pad as needed for drainage and runoff control.
- Top–dress with clean aggregate as needed.
- Immediately remove mud and sediment tracked or washed onto public roads.
- Flushing should only be used if the water from the construction drive can be conveyed into a sediment trap or basin.

TOPSOIL SALVAGE AND UTILIZATION

- Inspect daily.
- Check for damage to perimeter barrier; repair immediately.
- Check for erosion or damage to newly spread topsoil; repair immediately and revegetate.

TEMPORARY SEEDING

- Inspect within 24 hours of each rain event and at least once every seven calendar days.
- Check for erosion or movement of mulch and repair immediately.
- Monitor for erosion damage and adequate cover (80 percent density); reseed, fertilize, and apply mulch where necessary.
- If nitrogen deficiency is apparent, top–dress fall seeded wheat or rye seeding with 50 pounds per acre of nitrogen in February or March.

PERMANENT SEEDING

- Inspect within 24 hours of each rain event and at least once every seven calendar days until the vegetation is successfully established.
- Characteristics of a successful stand include vigorous dark green or bluishgreen seedlings with a uniform vegetative cover density of 90 percent or more.
- Check for erosion or movement of mulch.
- Repair damaged, bare, gullied, or sparsely vegetated areas and then fertilize, reseed, and apply and anchor mulch.
- If plant cover is sparse or patchy, evaluate the plant materials chosen, soil fertility, moisture condition, and mulch application; repair affected areas either by overseeding or preparing a new seedbed and reseeded. Apply and anchor mulch on the newly seeded areas.
- If vegetation fails to grow, consider soil testing to determine soil pH or nutrient deficiency problems. (Contact your soil and water conservation district or cooperative extension office for assistance.)
- If additional fertilization is needed to get a satisfactory stand, do so according to soil test recommendations.

EROSION CONTROL INSPECTION AND MAINTENANCE GUIDELINES (CONT.)

EROSION CONTROL MAINTENANCE GUIDELINES (CONT.)

PERMANENT SEEDING (CONT.)

- Add fertilizer the following growing season. Fertilize according to soil test recommendations.
- Fertilize turf areas annually. Apply fertilizer in a split application. For cool–season grasses, apply one–half of the fertilizer in late spring and onehalf in early fall. For warm–season grasses, apply one–third in early spring, one–third in late spring, and the remaining one–third in middle summer.

MULCHING

- Inspect within 24 hours of each rain event and at least once every seven calendar days.
- Check for erosion or movement of mulch; repair damaged areas, reseed, apply new mulch and anchor the mulch in place.
- Continue inspections until vegetation is firmly established.
- If erosion is severe or recurring, use erosion control blankets or other more substantial stabilization methods to protect the area.

EROSION CONTROL BLANKET

- Inspect within 24 hours of each rain event and at least once every seven calendar days.
- Check for erosion or displacement of the blanket.
- If any area shows erosion, pull back that portion of the blanket covering the eroded area, add soil and tamp, reseed the area, replace and staple the blanket.

TURF REINFORCEMENT MAT

- Inspect within 24 hours of each rain event and at least once every seven calendar days.
- Check for erosion or displacement/exposure of the mat.
- If a specific area shows erosion, add soil and restabilize.

ROCK CHECK DAM

- Inspect within 24 hours of each rain event and at least once every seven calendar days.
- If significant erosion occurs between dams, install an erosion–resistant liner in that portion of the channel.
- Remove accumulated sediment when it reaches one–half the height of the dam to maintain channel capacity, allow drainage through the dam, and prevent large flow from displacing sediment.
- Add riprap and aggregate as needed to maintain design height and cross section of the dams.
- When dams are no longer needed, remove the riprap and aggregate and stabilize the channel, using an erosion–resistant lining if necessary. (Riprap and aggregate from the dam may be removed or utilized to stabilize the channel.)

GEOTEXTILE FABRIC DROP INLET PROTECTION

- Inspect daily.
- Inspect geotextile fabric and make needed repairs immediately.
- Remove sediment from pool area to provide storage for the next storm event. Avoid damaging or undercutting fabric during sediment removal.
- When contributing drainage area has been stabilized, remove sediment, properly dispose of all construction material, grade area to the elevation of the storm drain inlet top, then stabilize immediately.

SILT FENCE

- Inspect within 24 hours of a rain event and at least once every seven calendar days.
- If fence fabric tears, starts to decompose, or in any way becomes ineffective, replace the affected portion immediately. **Note:** All repairs should meet specifications as outlined within this measure.
- Remove deposited sediment when it is causing the filter fabric to bulge or when it reaches one–half the height of the fence at its lowest point. When contributing drainage area has been stabilized, remove the fence and sediment deposits, grade the site to blend with the surrounding area, and stabilize.

PREFABRICATED CONCRETE WASHOUT SYSTEM/CONTAINER

- Inspect daily and after each storm event.
- Inspect the integrity of the overall structure including, where applicable, the containment system.
- Inspect the system for leaks, spills, and tracking of soil by equipment.
- Inspect the polyethylene lining for failure, including tears and punctures.
- Once concrete wastes harden, remove and dispose of the material.
- Excess concrete should be removed when the washout system reaches 50 percent of the design capacity. Use of the system should be discontinued until appropriate measures can be initiated to clean the structure. Prefabricated systems should also utilize this criterion, unless the manufacturer has alternate specifications.
- Upon removal of the solids, inspect the structure. Repair the structure as needed or construct a new system.
- Dispose of all concrete in a legal manner. Reuse the material on site, recycle, or haul the material to an approved construction/demolition landfill site. Recycling of material is encouraged. The waste material can be used for multiple applications including but not limited to roadbeds and building. The availability for recycling should be checked locally.
- The plastic liner should be replaced after every cleaning; the removal of material will usually damage the lining.
- Concrete washout systems are designed to promote evaporation. Prefabricated units are often pumped and the company supplying the unit provides this service.
- Inspect construction activities on a regular basis to ensure suppliers, contractors, and others are utilizing designated washout areas. If concrete waste is being disposed of improperly, identify the violators and take appropriate action.

DUST CONTROL

Minimizing wind erosion and controlling dust will be accomplished by one or more of the following methods:

- Covering 30% or more of the soil with a non–erodible material
- Roughening the soil to produces ridges (at least 6 inches) perpendicular to the prevailing wind.
- Frequent watering of excavation and fill areas
- Providing gravel or paving at entrance/exit drives, parking areas, and transit paths

REVISION



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Franklin, Indiana

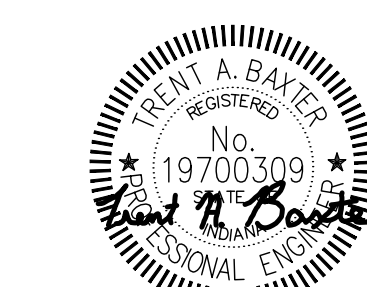
Erosion Control Details, Implementation
and Maintenance Guidelines

Job No.: 17023 Date Stamped: 01/21/2019

Drawn By: tab Checked By: tab Scale:

CAD FILE:

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SHEET TITLE:

C530

HANDICAP PARKING SIGNS

TYPICAL HEAVY DUTY ASPHALT PAVEMENT DETAIL
NO SCALE

REINFORCED CONCRETE PIPE (RCP)
TRENCH DETAIL

REINFORCED CONCRETE PIPE (RCP) TRENCH DETAIL

INTERNATIONAL HANDICAP DETAIL

NO SCALE

CURB "B" 18" INVERTED CONCRETE CURB & GUTTER DETAIL
NO SCALE

- ① 1 1/2" CONTROL JOINTS 10' O/C
- ② EXPANSION JOINTS 100' O/C

CURB "C" 18" CONCRETE CURB & GUTTER DETAIL

- ① 1 1/2" CONTROL JOINTS 10' O/C
- ② EXPANSION JOINTS 100' O/C

TYPICAL CONCRETE DUMPSTER PAVEMENT DETAIL
NO SCALE

TYPICAL CONCRETE TRANSFORMER PAD DETAIL
NO SCALE

CURB "A" 18" CONCRETE CURB & GUTTER DETAIL
NO SCALE

- ① 1 1/2" CONTROL JOINTS 10' O/C
- ② EXPANSION JOINTS 100' O/C

GREATER THAN 5' FROM EDGE OF PAVEMENT

- NOTE:**
ALL BEDDING AND INITIAL BACKFILL
SHALL BE INSTALLED IN 6"
TO 12" BALANCED LIFTS.
- A MINIMUM 9" CLEARANCE SHALL
BE PROVIDED ON EACH SIDE OF
THE INSTALLED PIPE.
- LEGEND**
- Bc = OUTSIDE DIAMETER
D = INSIDE DIAMETER
d = DEPTH OF BEDDING
MATERIAL BELOW PIPE

WITHIN 5' OF EDGE OF PAVEMENT

- NOTE:**
ALL BEDDING AND INITIAL BACKFILL
SHALL BE INSTALLED IN 6"
TO 12" BALANCED LIFTS.
- A MINIMUM 9" CLEARANCE SHALL
BE PROVIDED ON EACH SIDE OF
THE INSTALLED PIPE.
- LEGEND**
- Bc = OUTSIDE DIAMETER
D = INSIDE DIAMETER
d = DEPTH OF BEDDING
MATERIAL BELOW PIPE

- NOTES:
1. ALL MATERIAL AND CONSTRUCTION TO CONFORMANCE WITH INDIANA DEPT. OF TRANSPORTATION (INDOT) SPECIFICATIONS.
 2. POURED BENCHWALL AND PIPE CONNECTION VARY WITH BOX TYPE SPECIFIED.
 3. HEIGHT DIMENSIONS MAY VARY AS REQUIRED BY SITE CONDITIONS.

Precast Concrete Box Inlet

NOTE: ALL MANHOLES AND INLETS SHALL BE PLACED ON NO LESS THAN 6 INCHES OF STONE.

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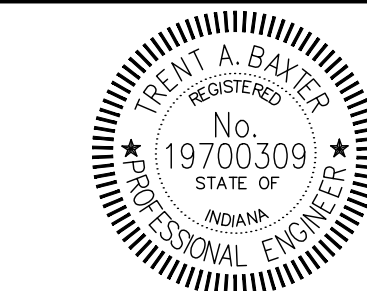
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Franklin, Indiana

General Sitework Details

Job No.:	Date Stamped:
17023	01/21/2019

Drawn By: caw	Checked By: tab	Scale:
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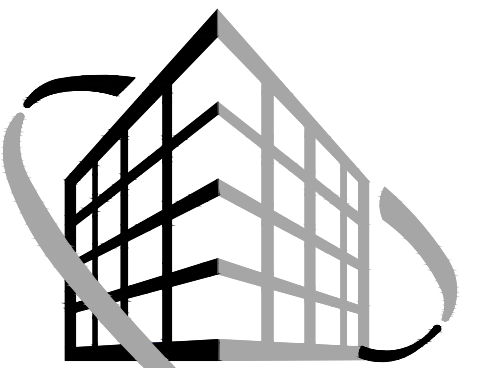
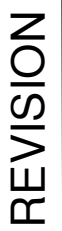


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C600



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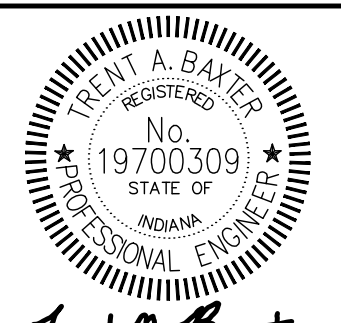
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Franklin Gateway Development
Franklin, Indiana

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Job No.:	Date Stamped:
17023	01/21/2019

Drawn By:	Checked By:	Scale:
caw	tab	

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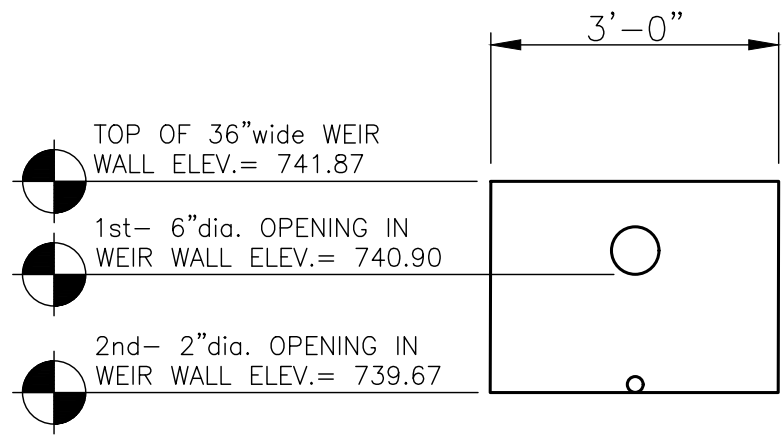


Trent A. Baxter
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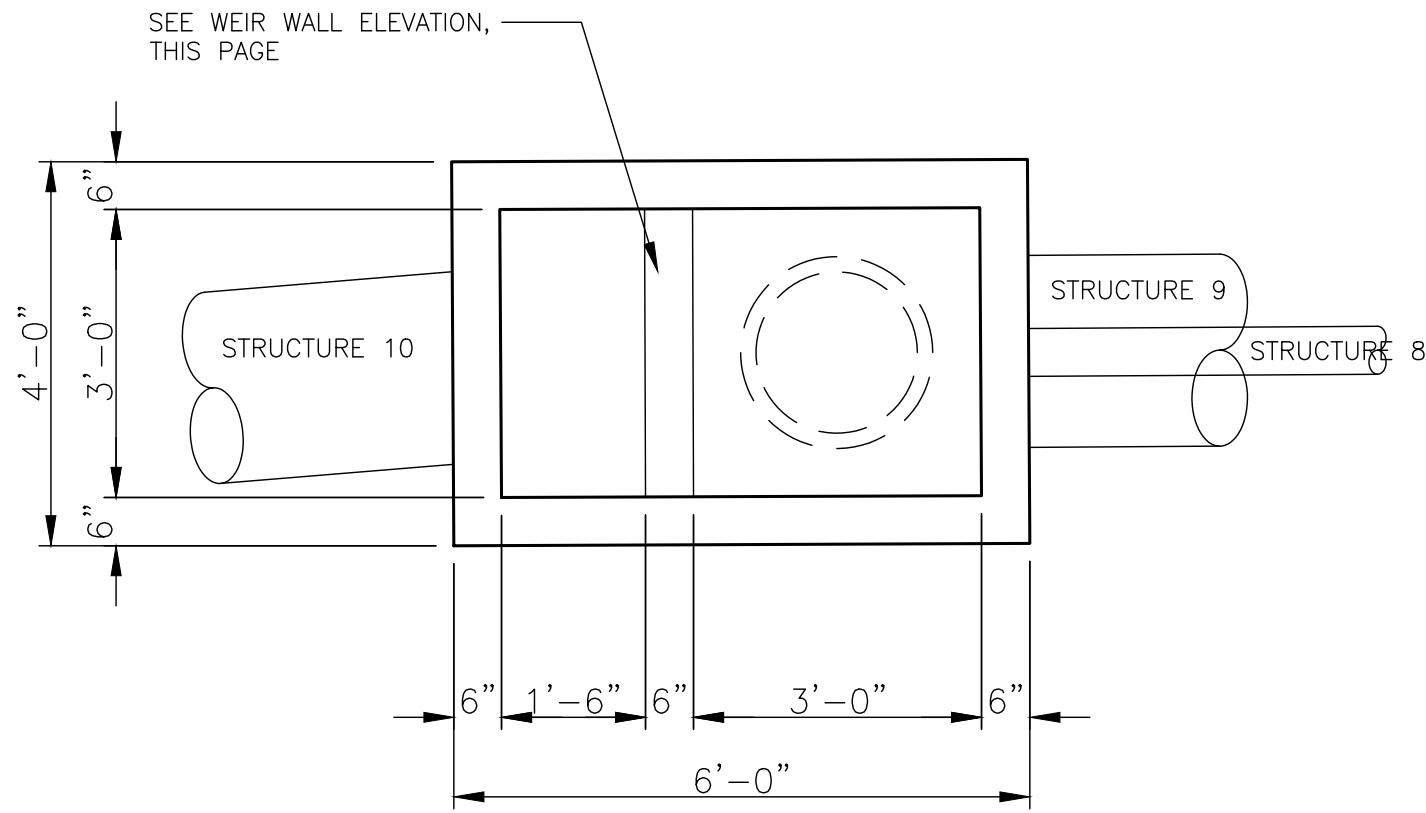
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C610

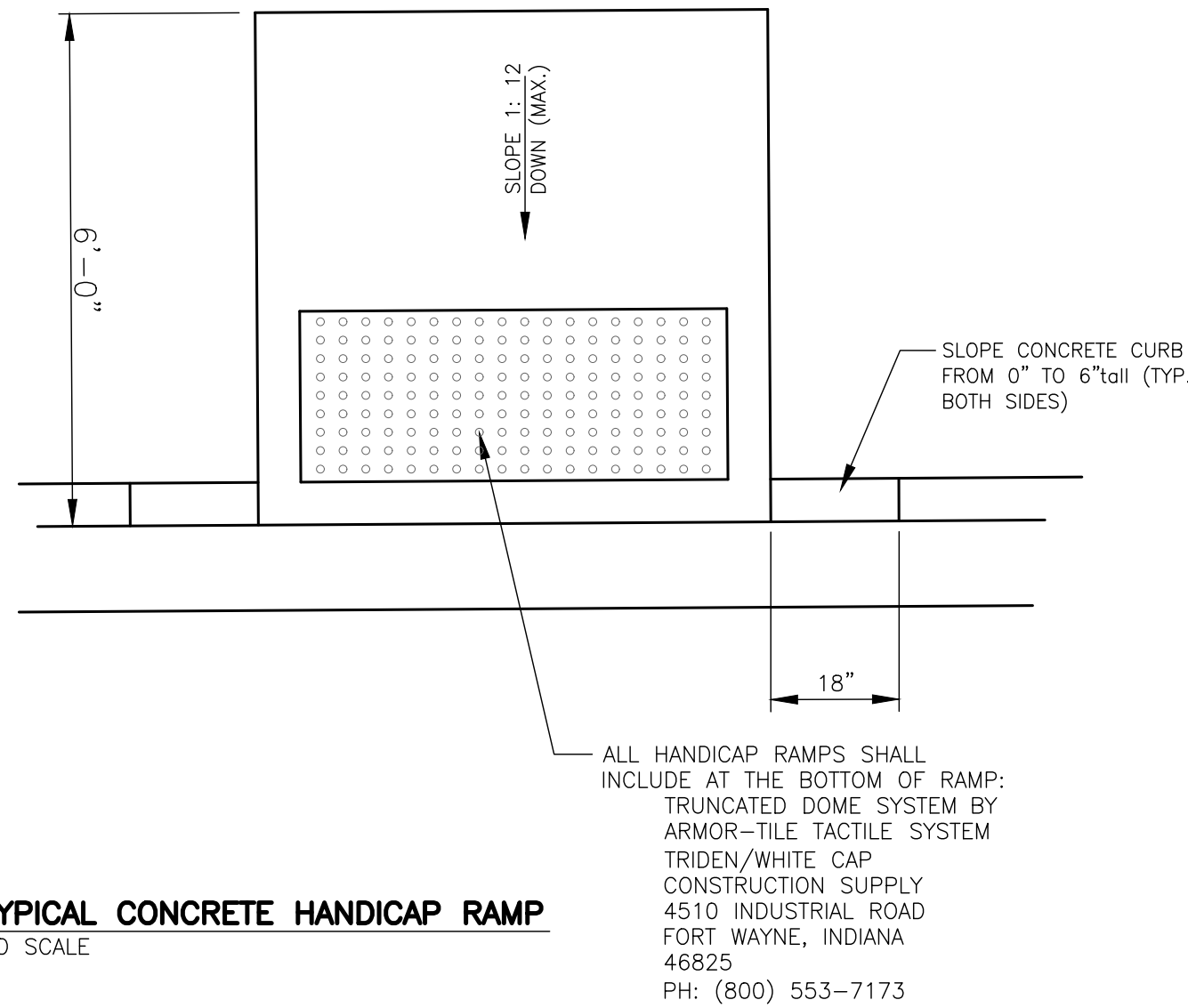


WEIR WALL ELEVATION
NO SCALE

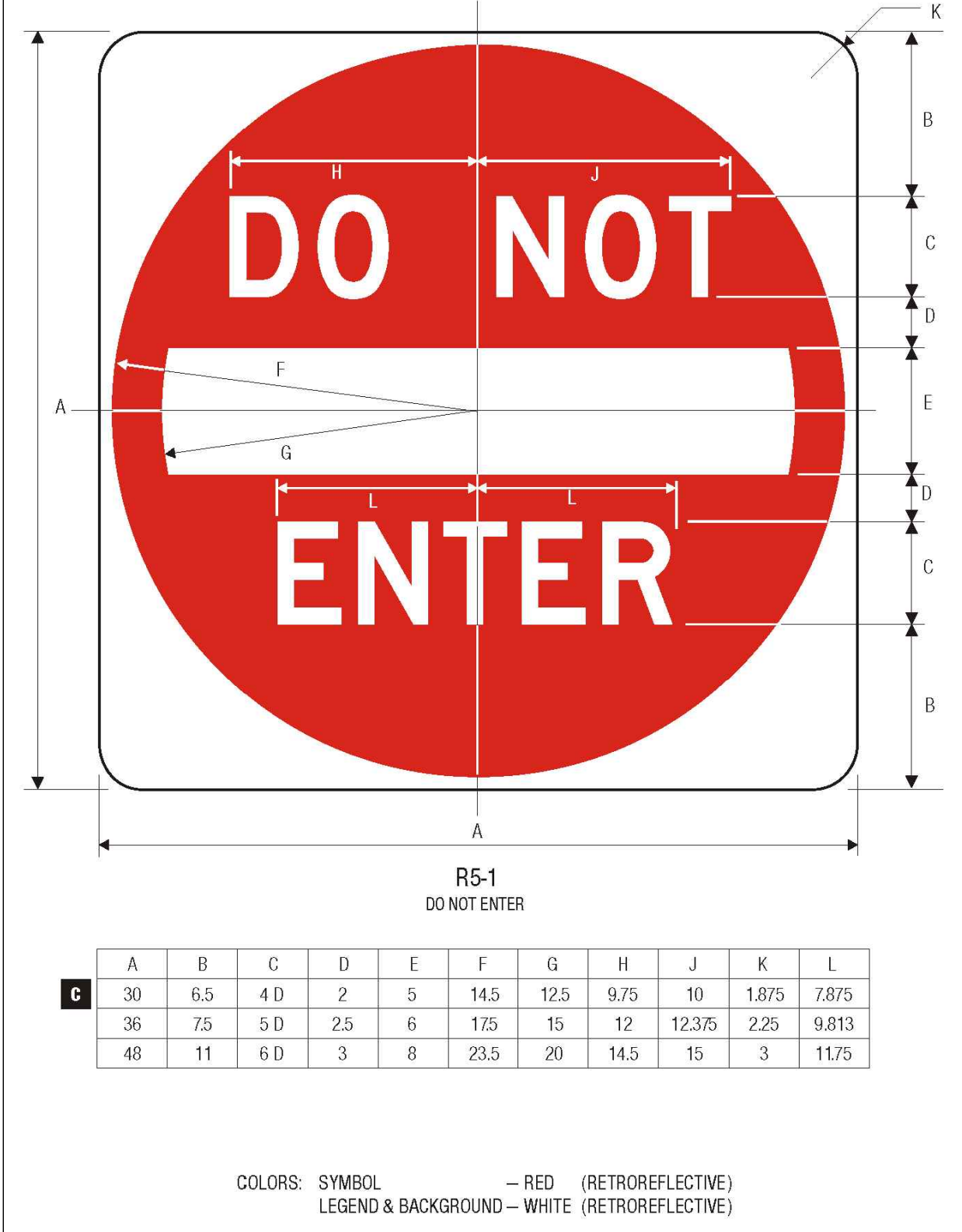


- STRUCTURE No.g
NEW 3.0'x 5.0' PRECAST CONCRETE STORM
MANHOLE WITH SOLID LID
CASTING: NEENAH R-1733
T.C.= 744.80
EAST INV. IN 4"pvc (PERFORATED)= 739.67
EAST INV. IN 24"dia. N12 hdpe= 740.75
WEST INV. OUT 24"dia. N12 hdpe= 739.67
TOP OF 36"wide WEIR WALL ELEV.= 741.87
1st- 6"dia. OPENING IN WEIR WALL ELEV.= 740.90
2nd- 2"dia. OPENING IN WEIR WALL ELEV.= 739.67

STORMWATER STRUCTURE No.g (CONTROL STRUCTURE)
NO SCALE

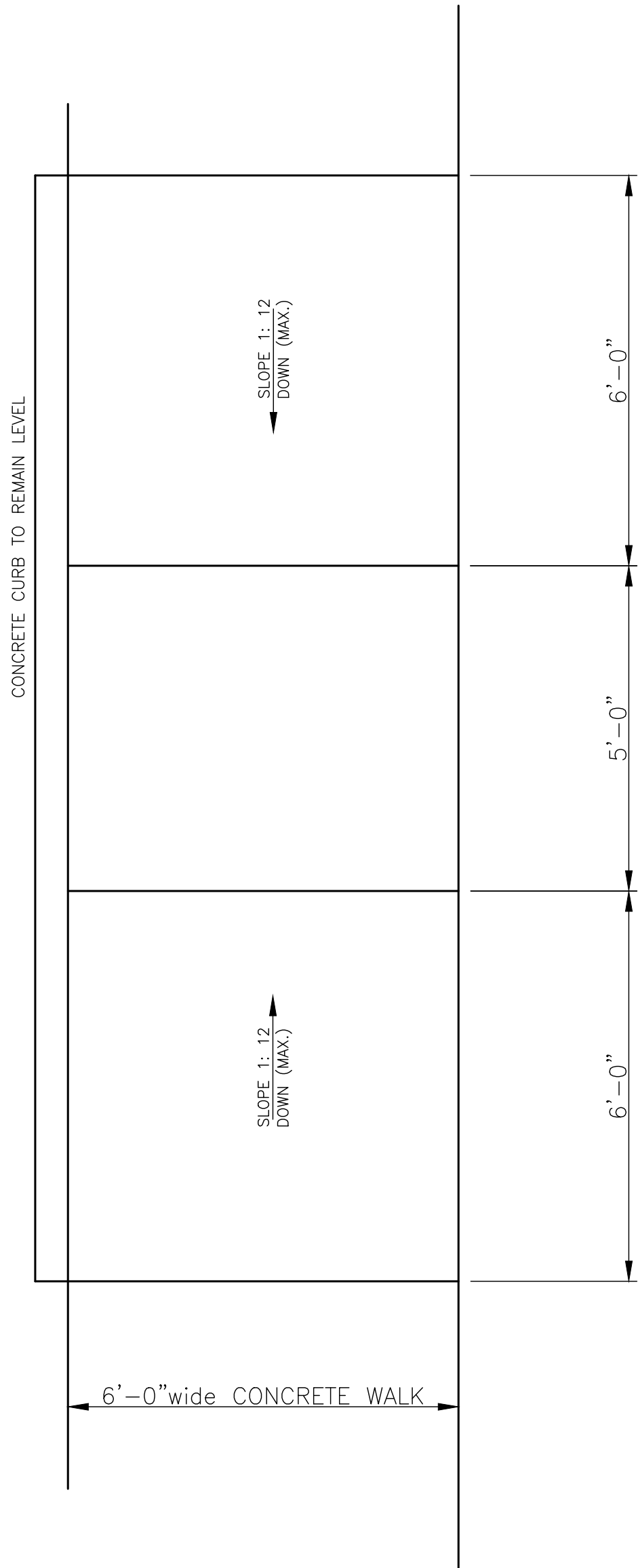


TYPICAL CONCRETE HANDICAP RAMP
NO SCALE

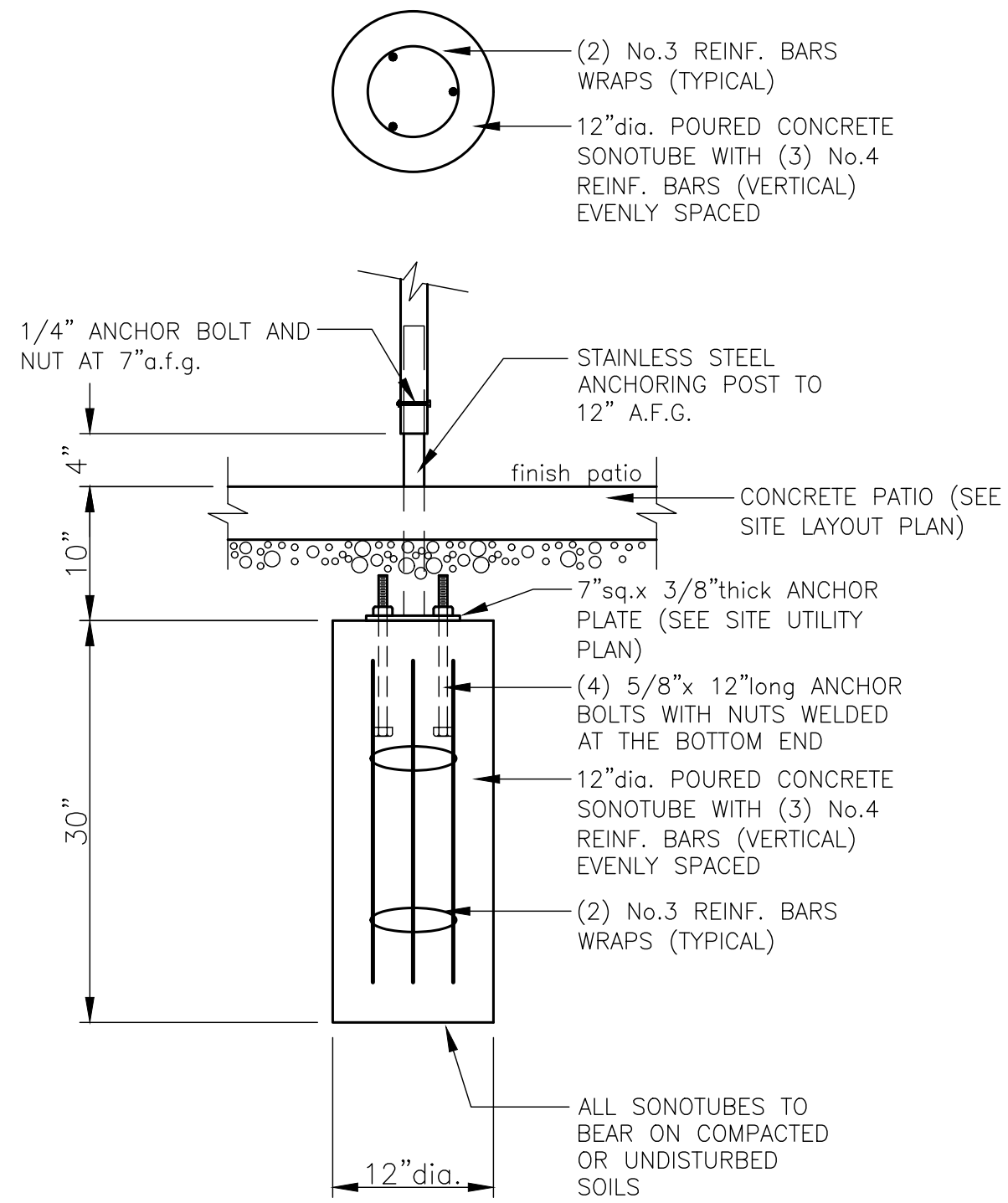


A	B	C	D	E	F	G	H	J	K	L
30	6.5	4 D	2	5	14.5	12.5	9.75	10	1.875	7.875
36	7.5	5 D	2.5	6	17.5	15	12	12.375	2.25	9.813
48	11	6 D	3	8	23.5	20	14.5	15	3	11.75

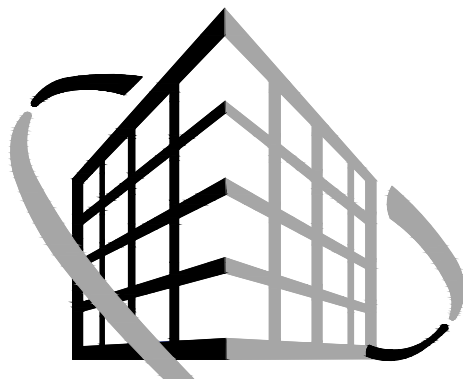
COLORS: SYMBOL — RED (RETROREFLECTIVE)
LEGEND & BACKGROUND — WHITE (RETROREFLECTIVE)



CONCRETE HANDICAP RAMPS AT HANDICAP PARKING
NO SCALE



PATIO LIGHT POLE CONCRETE BASE DETAIL
SCALE: no scale



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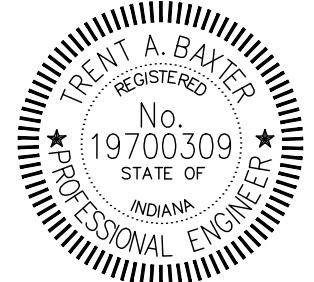
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Franklin, Indiana

General Sitework Details

Job No.: 17023	Date Stamped: 01/21/2019
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Scale: 1/2" = 1'-0"	

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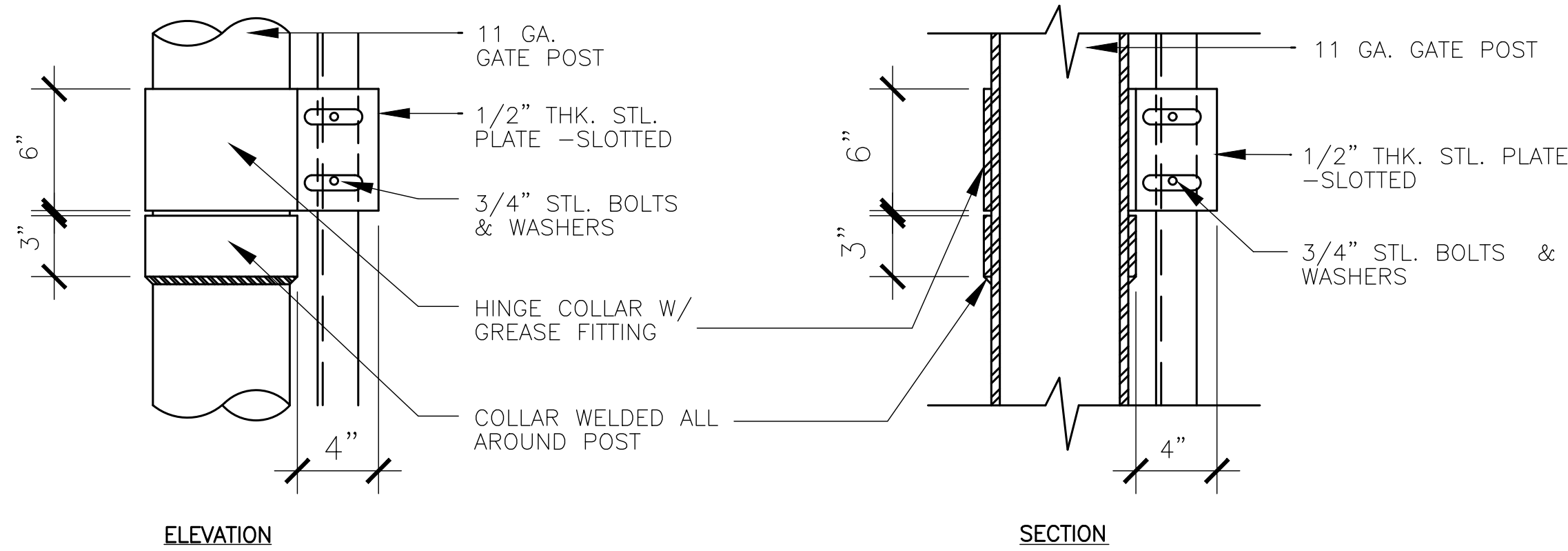


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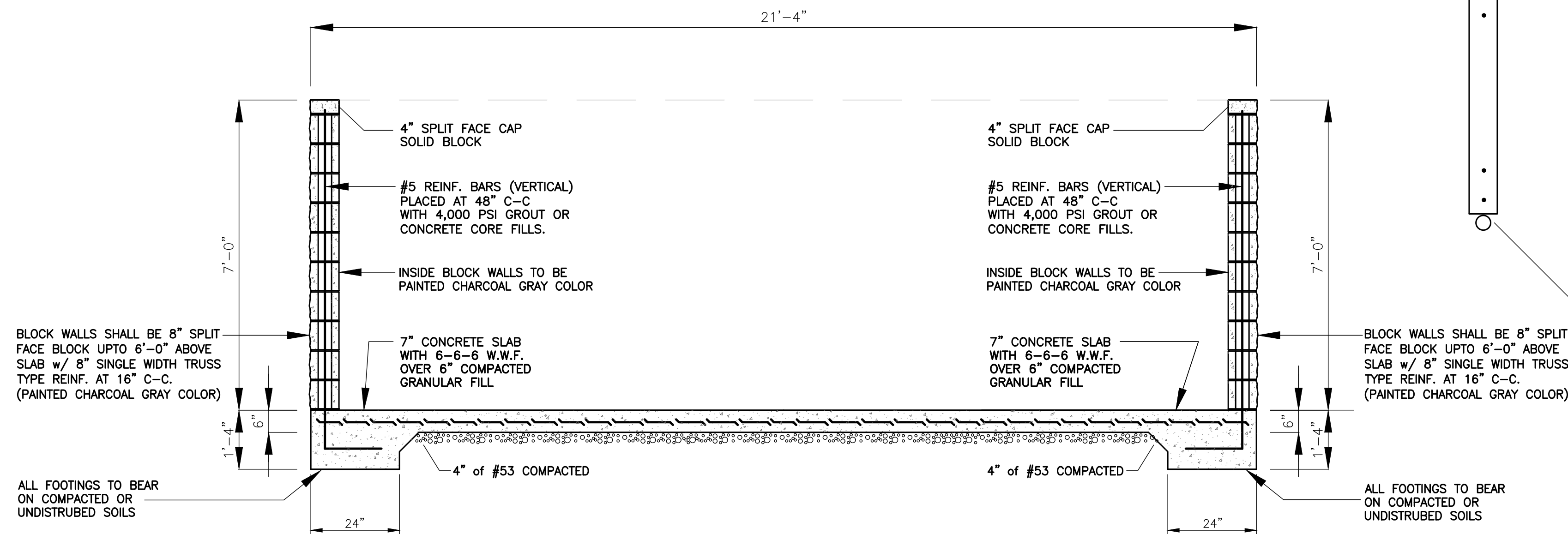
C620



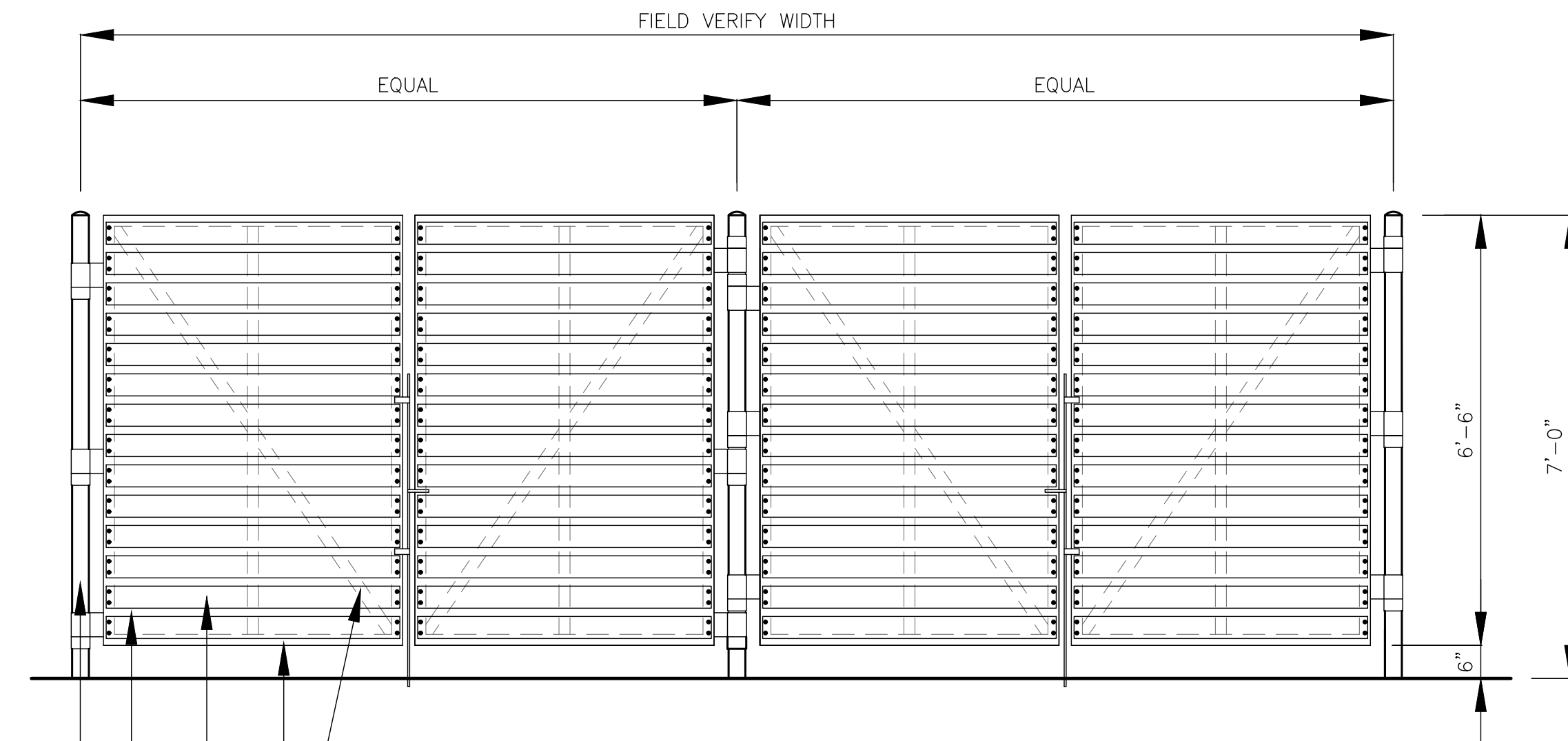
HINGE CONNECTION DETAIL



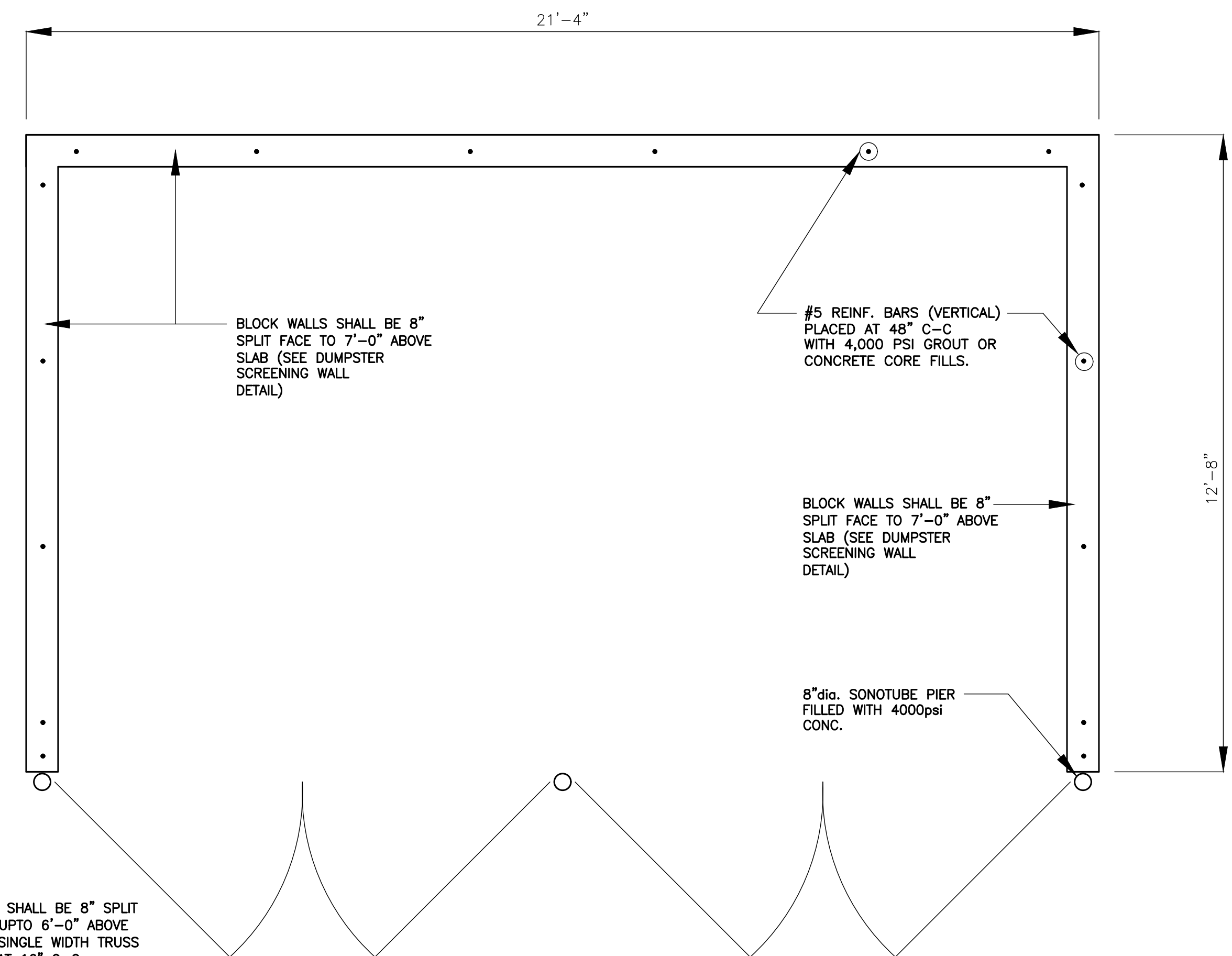
TREX SCREENING WALL ELEVATION
NO SCALE



DUMPSTER SCREENING WALL ELEVATION
SCALE: 1/2" = 1'-0"

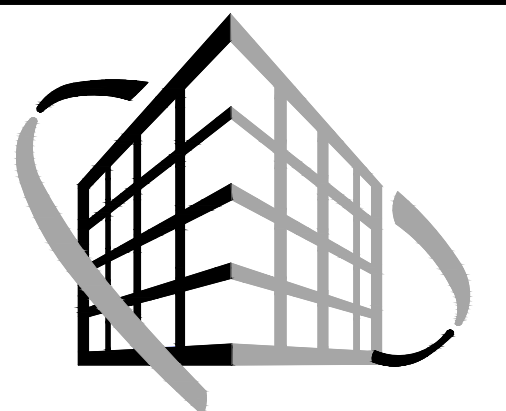


DUMPSTER GATE ELEVATION



DUMPSTER SCREENING WALLS PLAN
SCALE: 1/2" = 1'-0"

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C630



STORMTECH CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE STORMTECH SC-740 OR SC-310.
2. CHAMBERS SHALL BE MANUFACTURED FROM VIRGIN POLYPROPYLENE OR POLYETHYLENE RESINS.
3. CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORT PANELS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
4. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
5. CHAMBERS SHALL MEET ASTM F2922 (POLYETHYLENE) OR ASTM F2418-16 (POLYPROPYLENE), "STANDARD SPECIFICATION FOR THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
6. CHAMBERS SHALL BE DESIGNED AND ALLOWABLE LOADS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
7. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. THE CHAMBER MANUFACTURER SHALL SUBMIT THE FOLLOWING UPON REQUEST TO THE SITE DESIGN ENGINEER FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE:
 - a. A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY AASHTO FOR THERMOPLASTIC PIPE.
 - b. A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET. THE 50 YEAR CREEP MODULUS DATA SPECIFIED IN ASTM F2418 OR ASTM F2922 MUST BE USED AS PART OF THE AASHTO STRUCTURAL EVALUATION TO VERIFY LONG-TERM PERFORMANCE.
 - c. STRUCTURAL CROSS SECTION DETAIL ON WHICH THE STRUCTURAL EVALUATION IS BASED.
8. CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF THE SC-310/SC-740 SYSTEM

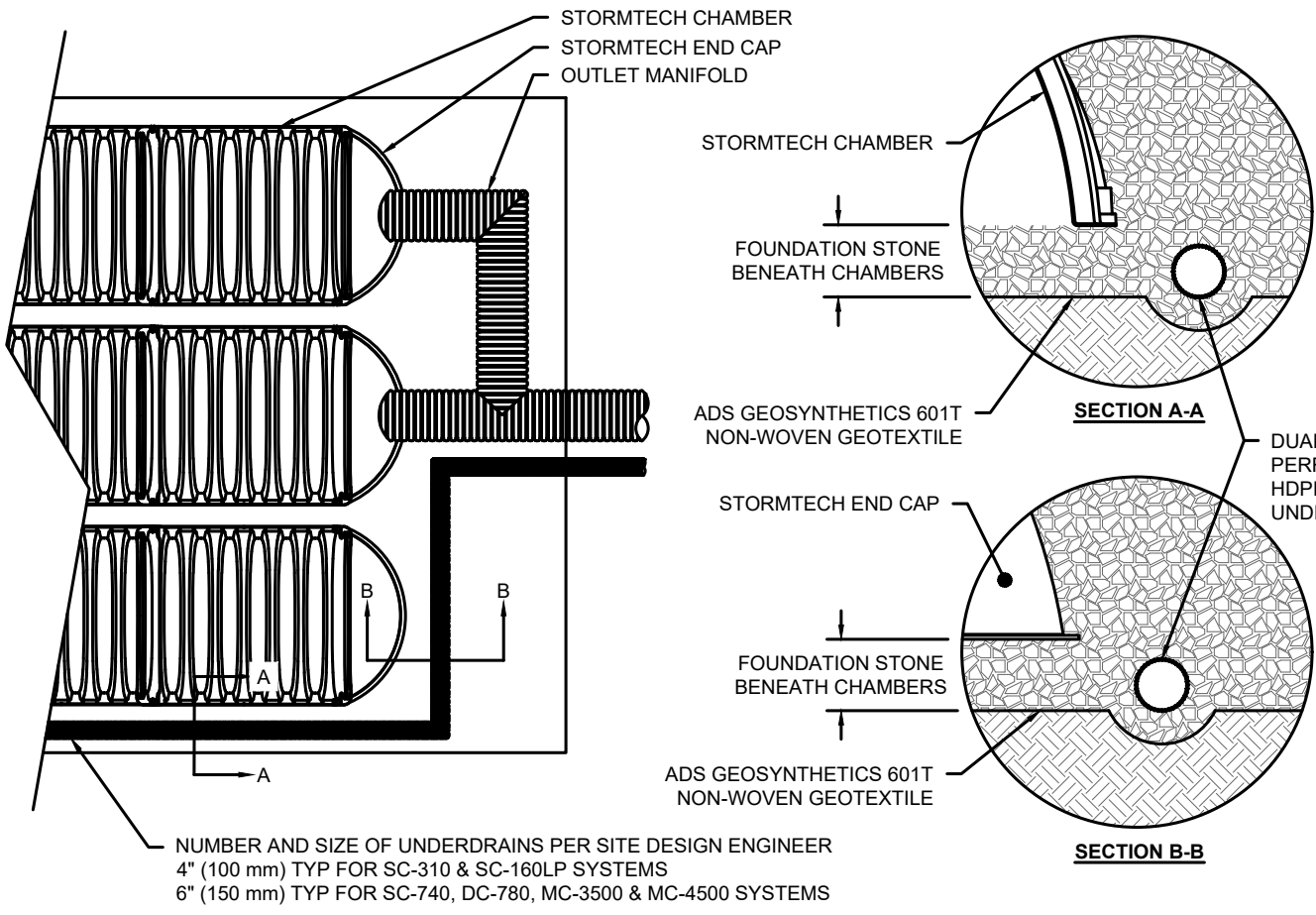
1. STORMTECH SC-310 & SC-740 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
2. STORMTECH SC-310 & SC-740 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
3. CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR AN EXCAVATOR SITUATED OVER THE CHAMBERS. STORMTECH RECOMMENDS 3 BACKFILL METHODS:
 - STONESHOTTER LOCATED OFF THE CHAMBER BED.
 - BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
 - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
4. THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.
5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
6. MAINTAIN MINIMUM - 6" (150 mm) SPACING BETWEEN THE CHAMBER ROWS.
7. EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE 3/4-2" (20-50 mm).
8. THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIALS BEARING CAPACITIES TO THE SITE DESIGN ENGINEER.
9. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

NOTES FOR CONSTRUCTION EQUIPMENT

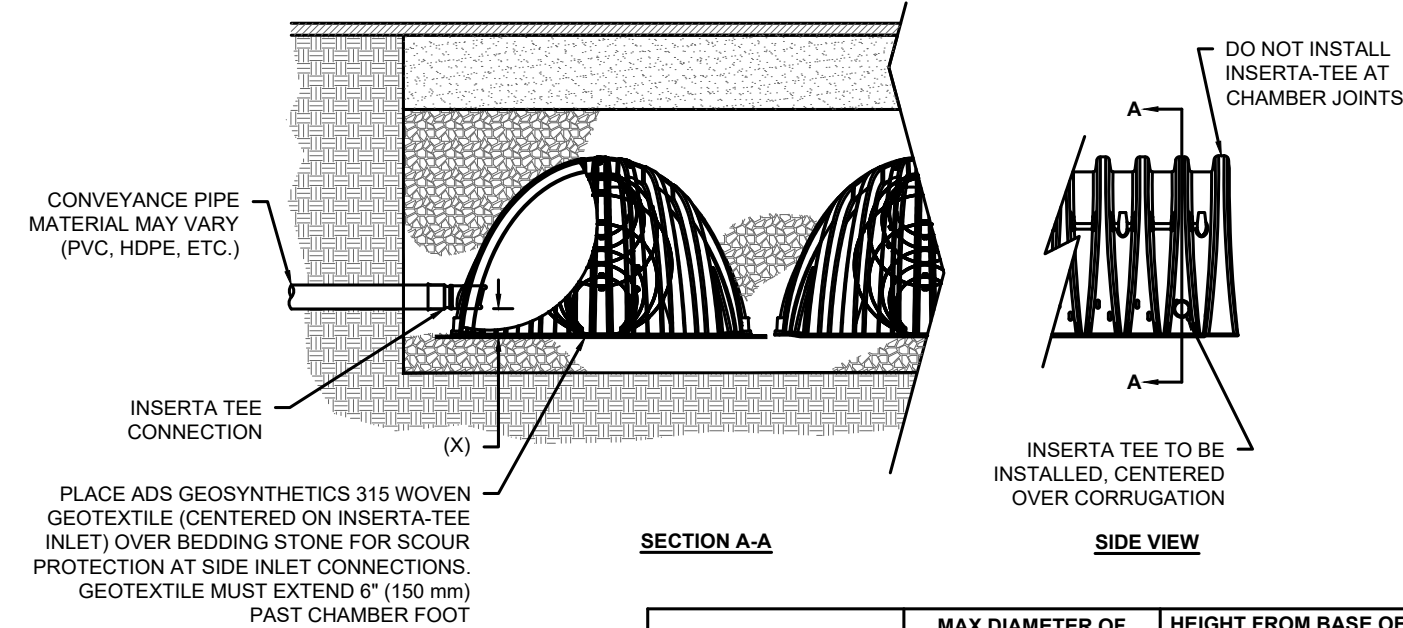
1. STORMTECH SC-310 & SC-740 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
2. THE USE OF CONSTRUCTION EQUIPMENT OVER SC-310 & SC-740 CHAMBERS IS LIMITED:
 - NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS.
 - NO RUBBER Tired LOADERS, DUMP TRUCKS, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
 - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
3. FULL 36" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO THE CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-888-892-0894 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

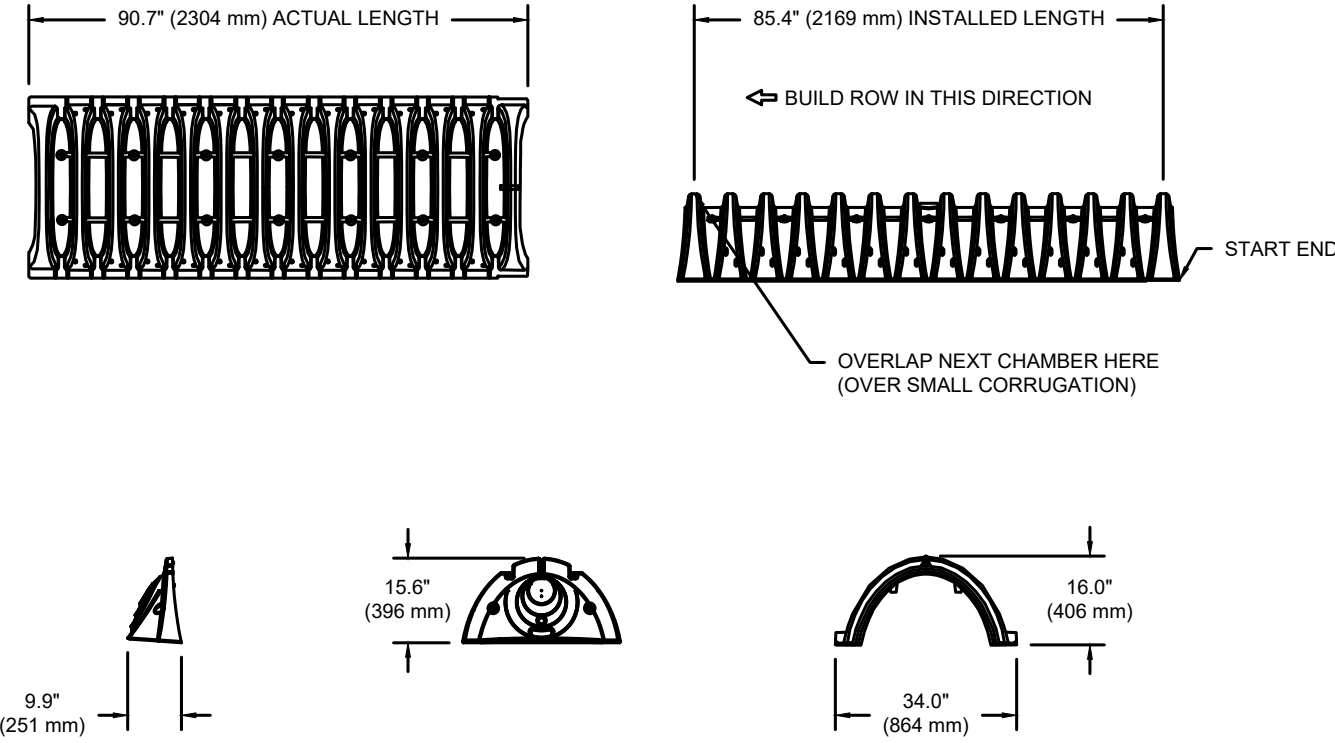


UNDERDRAIN DETAIL



CHAMBER	MAX DIAMETER OF INSERTA TEE	HEIGHT FROM BASE OF CHAMBER (X)
SC-310	6" (150 mm)	4" (100 mm)
SC-740	10" (250 mm)	4" (100 mm)
DC-780	10" (250 mm)	4" (100 mm)
MC-3500	12" (300 mm)	6" (150 mm)
MC-3500	12" (300 mm)	6" (200 mm)
INSERTA TEE FITTINGS AVAILABLE FOR SDR 35, SDR 35, SCH 40 IPS, GASKETED & SOLVENT WELD, N-12, HP STORM, C-900 OR DUCTILE IRON		

NOTE: PART NUMBERS WILL VARY BASED ON INLET PIPE MATERIALS. CONTACT STORMTECH FOR MORE INFORMATION.



NOMINAL CHAMBER SPECIFICATIONS	34.0" X 16.0" X 85.4"	(864 mm X 406 mm X 2169 mm)
SIZE (W X H X INSTALLED LENGTH)	14.7 CUBIC FEET	(0.42 m³)
CHAMBER STORAGE	31.0 CUBIC FEET	(0.88 m³)
MINIMUM INSTALLED STORAGE*	35.0 lbs.	(16.8 kg)

*ASSUMES 6" (152 mm) ABOVE, BELOW, AND BETWEEN CHAMBERS

PRE-FAB STUBS AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"
PRE-FAB STUBS AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"
PRE-CORED END CAPS END WITH "PC"

PART #	STUB	A	B	C
SC310EPE00T / SC310EPE00TPC	6" (150 mm)	9.6" (244 mm)	5.8" (147 mm)	---
SC310EPE00B / SC310EPE00BPC	---	---	---	0.5" (13 mm)
SC310EPE00T / SC310EPE00TPC	8" (200 mm)	11.9" (302 mm)	3.5" (89 mm)	---
SC310EPE00B / SC310EPE00BPC	---	---	---	0.6" (15 mm)
SC310EPE10T / SC310EPE10TPC	10" (250 mm)	12.7" (323 mm)	1.4" (36 mm)	---
SC310EPE10B / SC310EPE10BPC	---	---	---	0.7" (18 mm)
SC310EPE12B	12" (300 mm)	13.5" (343 mm)	---	0.9" (23 mm)

ALL STUBS, EXCEPT FOR THE SC310EPE12B ARE PLACED AT BOTTOM OF END CAP SUCH THAT THE OUTSIDE DIAMETER OF THE STUB IS FLUSH WITH THE BOTTOM OF THE END CAP. FOR ADDITIONAL INFORMATION CONTACT STORMTECH AT 1-888-892-0894.

* FOR THE SC310EPE12B THE 12" (300 mm) STUB LIES BELOW THE BOTTOM OF THE END CAP APPROXIMATELY 0.25" (6 mm). BACKFILL MATERIAL SHOULD BE REMOVED FROM BELOW THE N-12 STUB SO THAT THE FITTING SITS LEVEL.

NOTE: ALL DIMENSIONS ARE NOMINAL.

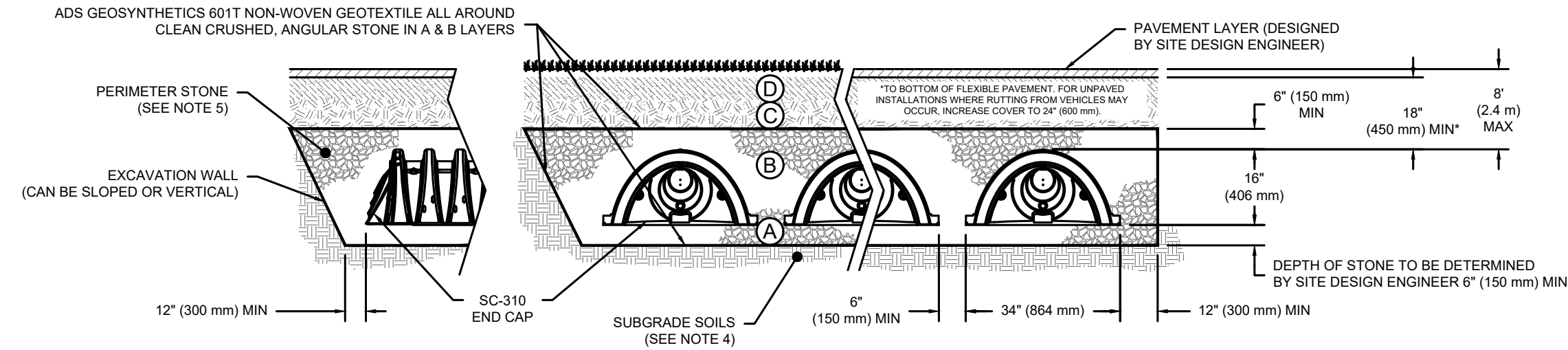
INSERTA-TEE SIDE INLET DETAIL

SC-310 TECHNICAL SPECIFICATIONS

ACCEPTABLE FILL MATERIALS: STORMTECH SC-310 CHAMBER SYSTEMS

MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 18" (450 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	AASHTO M145 ¹ A-1, A-2-4, A-3 OR AASHTO M43 ³ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	AASHTO M43 ³ 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	AASHTO M43 ³ 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. ^{1,2}

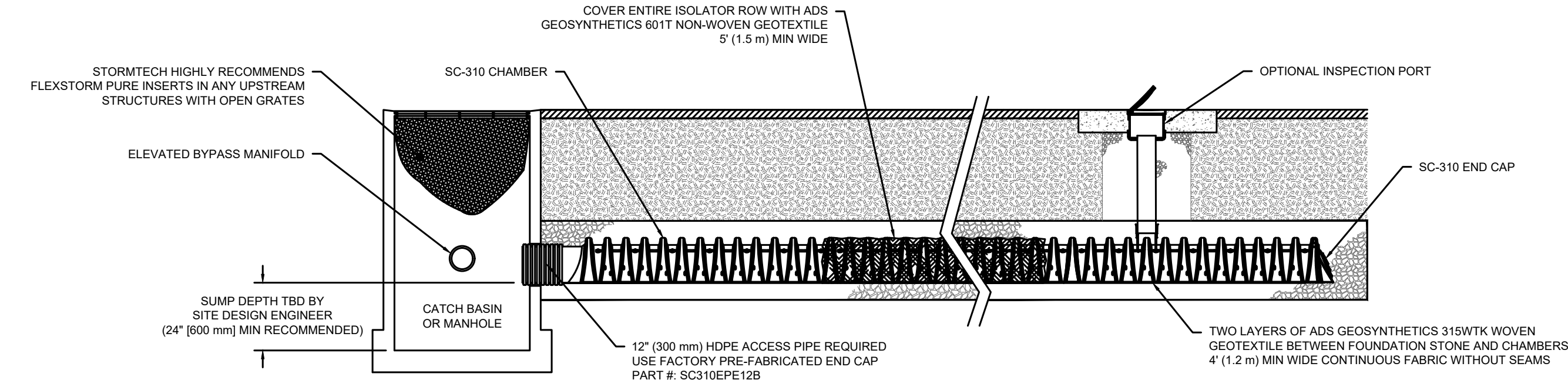
PLEASE NOTE:
1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'X' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.



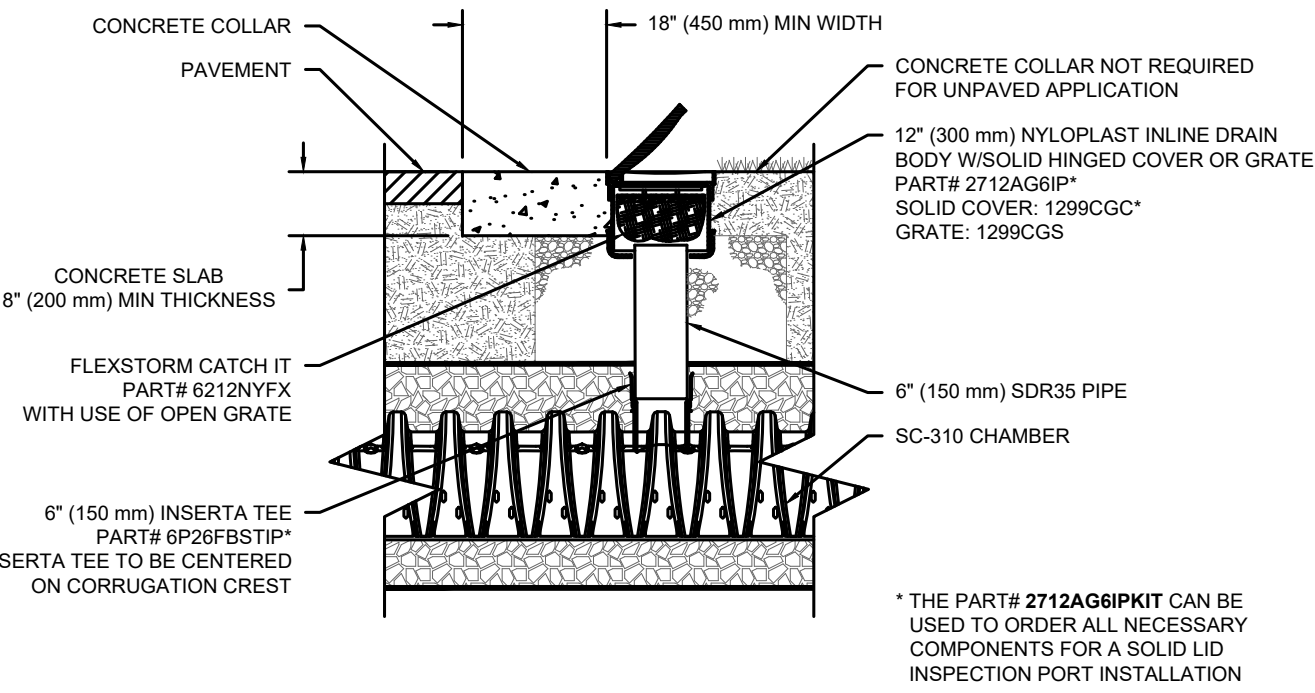
NOTES:

1. SC-310 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS", OR ASTM F2922 "STANDARD SPECIFICATION FOR POLYETHYLENE (PE) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-310 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. "ACCEPTABLE FILL MATERIALS" TABLE ABOVE PROVIDES MATERIAL LOCATIONS, DESCRIPTIONS, GRADATIONS, AND COMPACTION REQUIREMENTS FOR FOUNDATION, EMBEDMENT, AND FILL MATERIALS.
4. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
5. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
6. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.

SC-310 CROSS SECTION DETAIL



SC-310 ISOLATOR ROW DETAIL



SC-310 6" (150 mm) INSPECTION PORT DETAIL

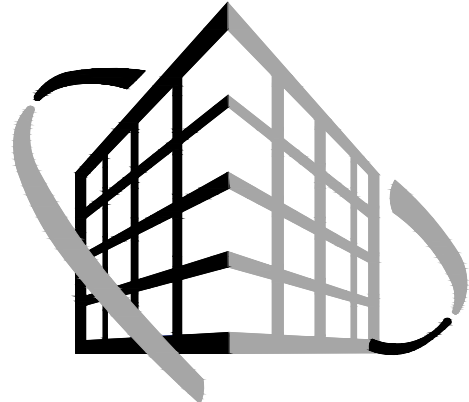
INSPECTION & MAINTENANCE

- STEP 1) INSPECT ISOLATOR ROW FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
 - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
 - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
 - A.3. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW THROUGH OUTLET PIPE
 - A.4. LOWER A CAMERA INTO ISOLATOR ROW FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
 - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
 - B. ALL ISOLATOR ROWS
 - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW
 - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW THROUGH OUTLET PIPE
 - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
 - i) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
 - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45° (1.1 m) OR MORE IS PREFERRED
 - B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
 - C. VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

REVISION



VERSATILE
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GROUP, LLC.

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New Whiteland, Indiana 46184
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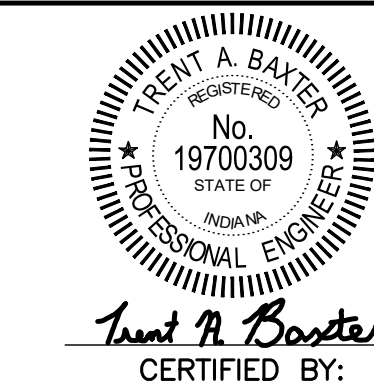
Starbucks
Franklin Gateway Development
Franklin, Indiana

General Sitework Details

Job No.: 17023 Date Stamped: 01/21/2019

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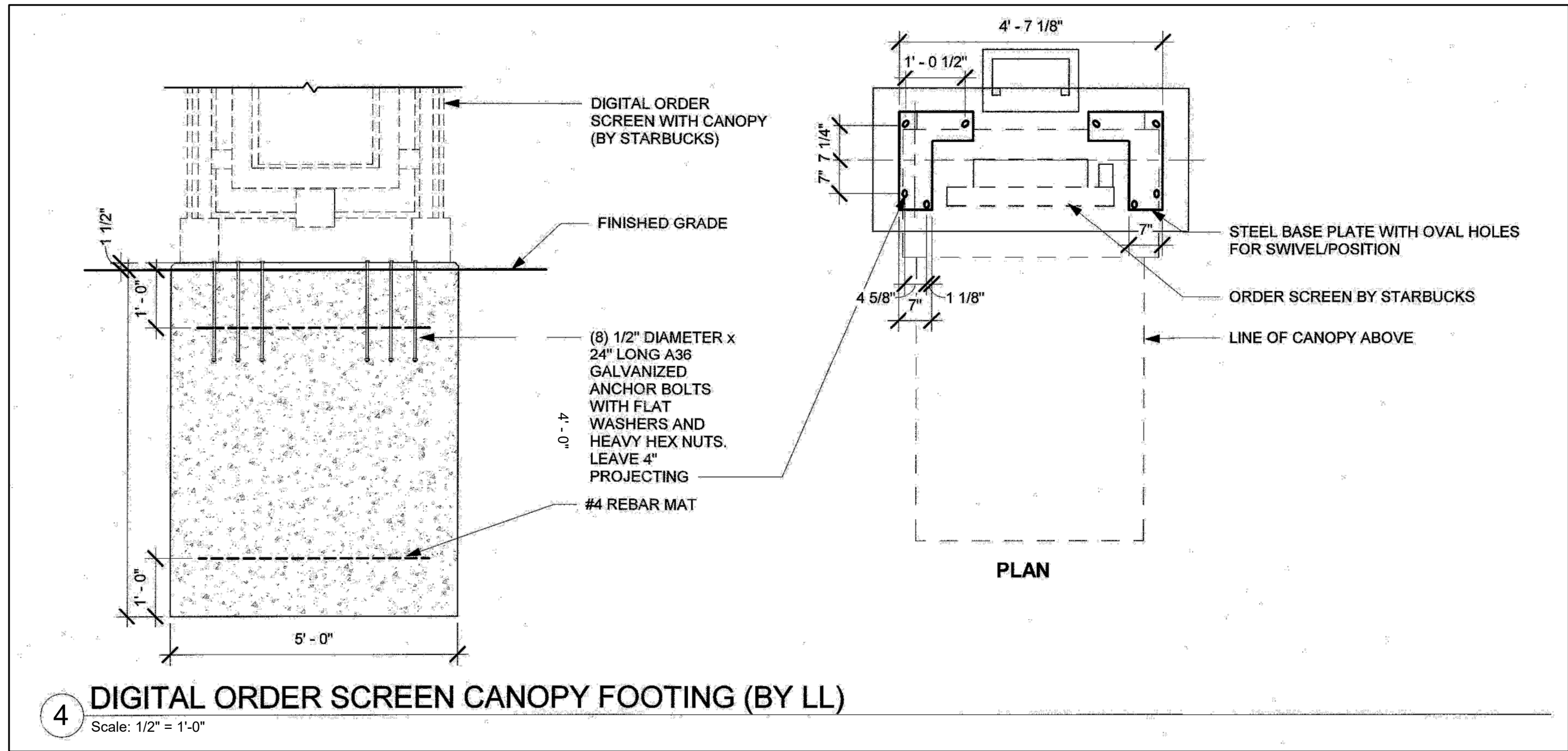
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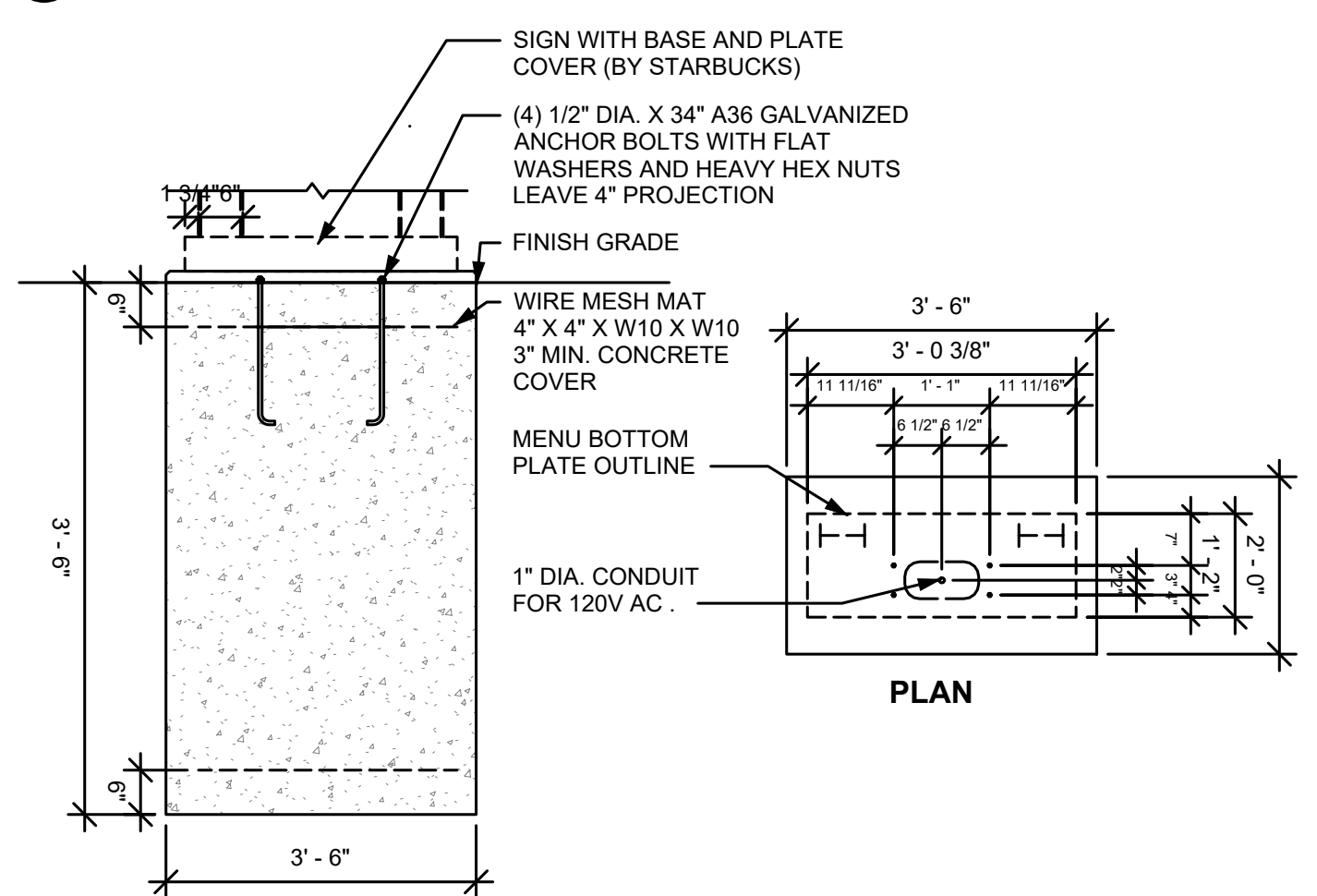
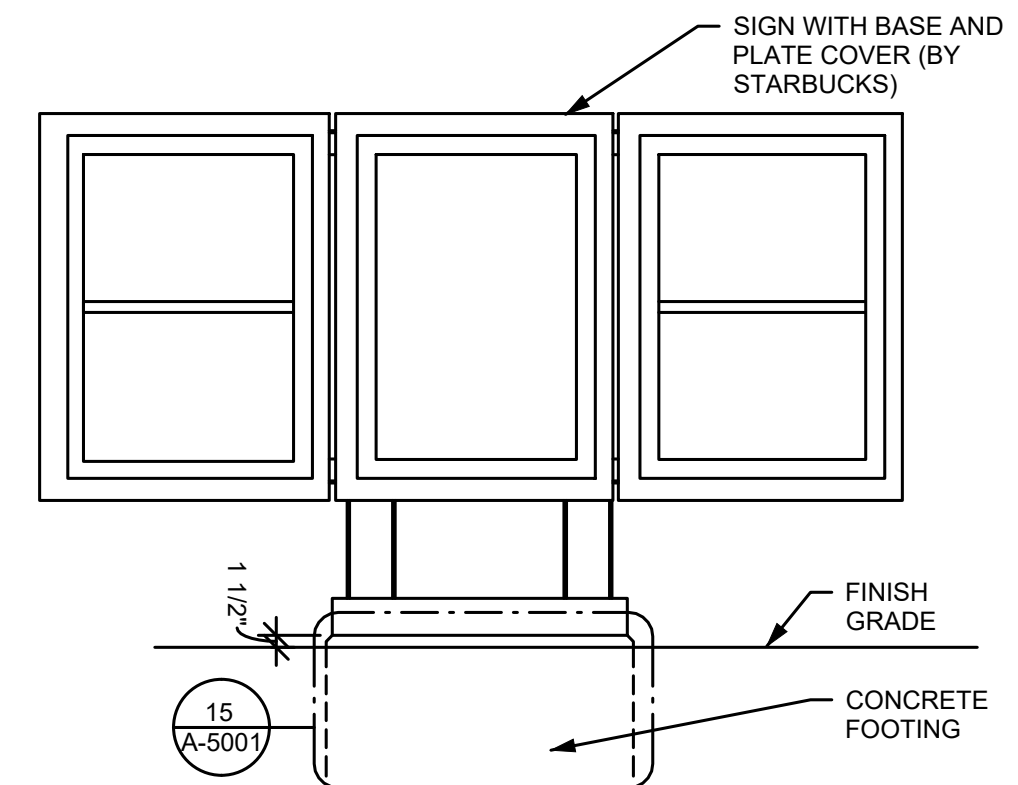
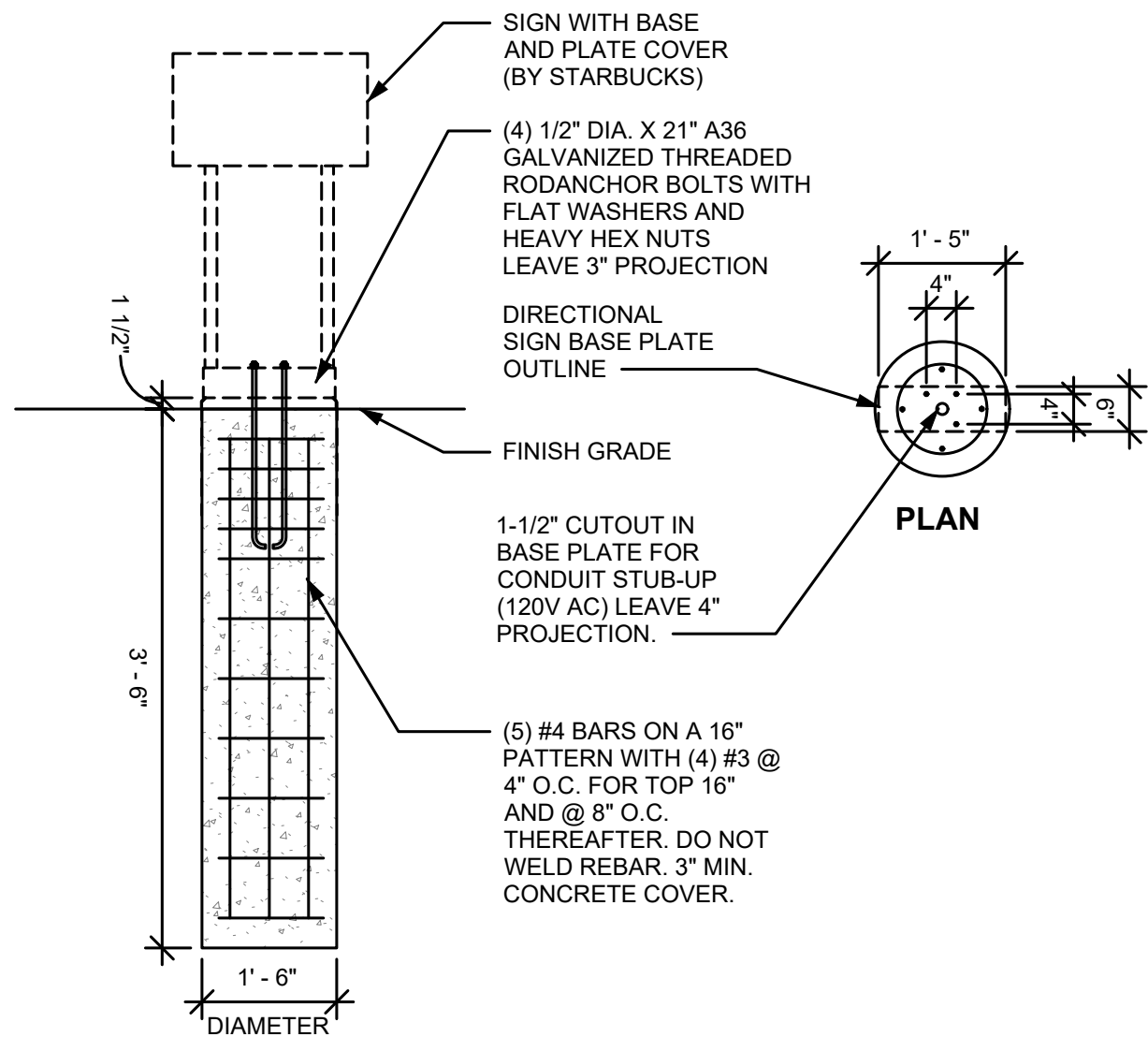
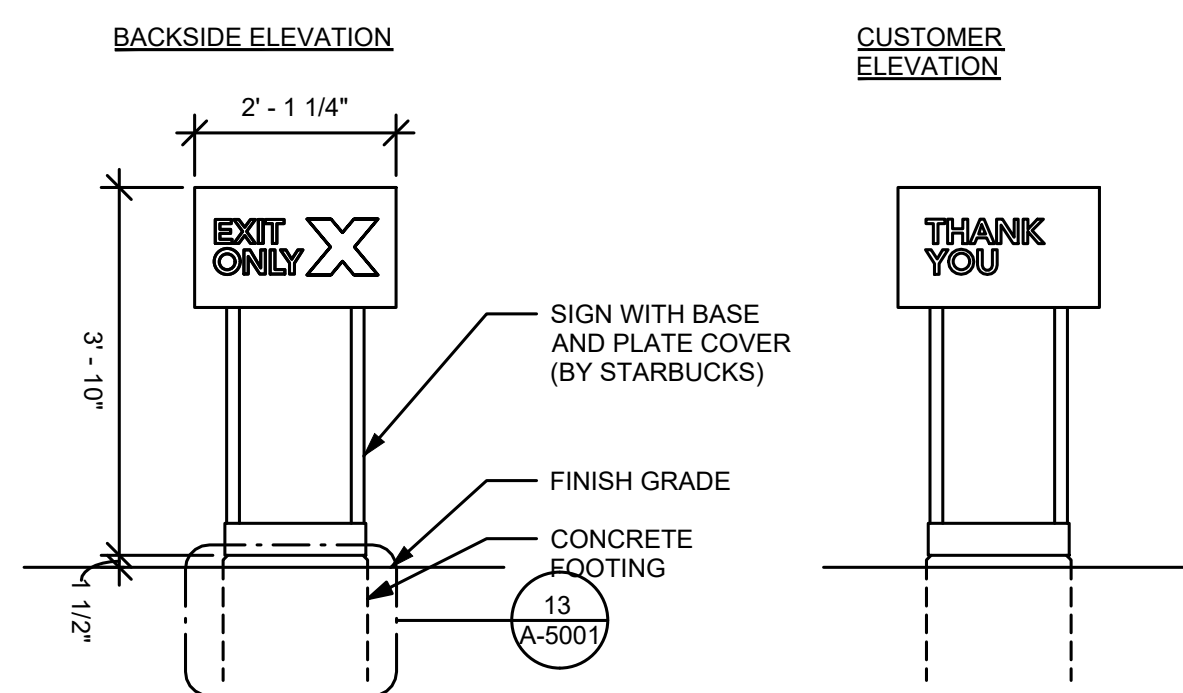
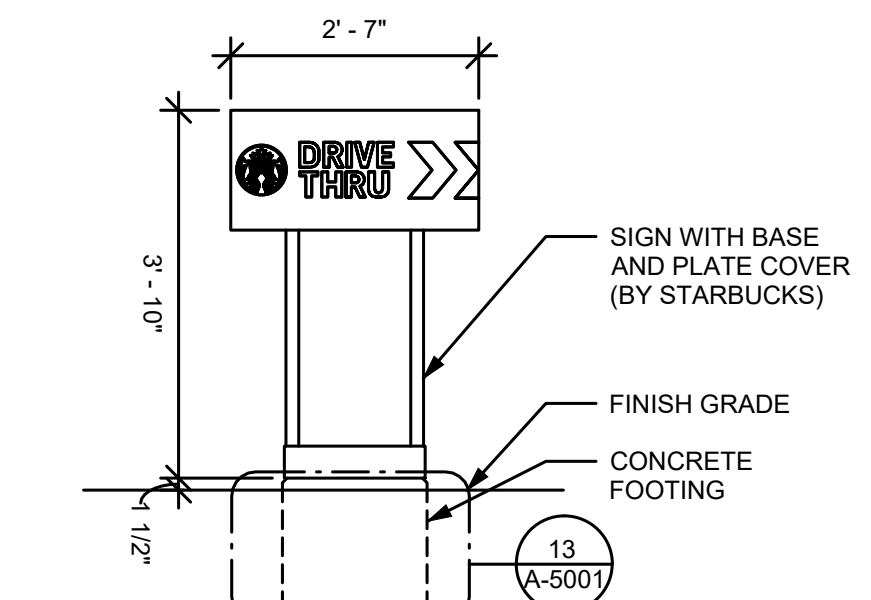
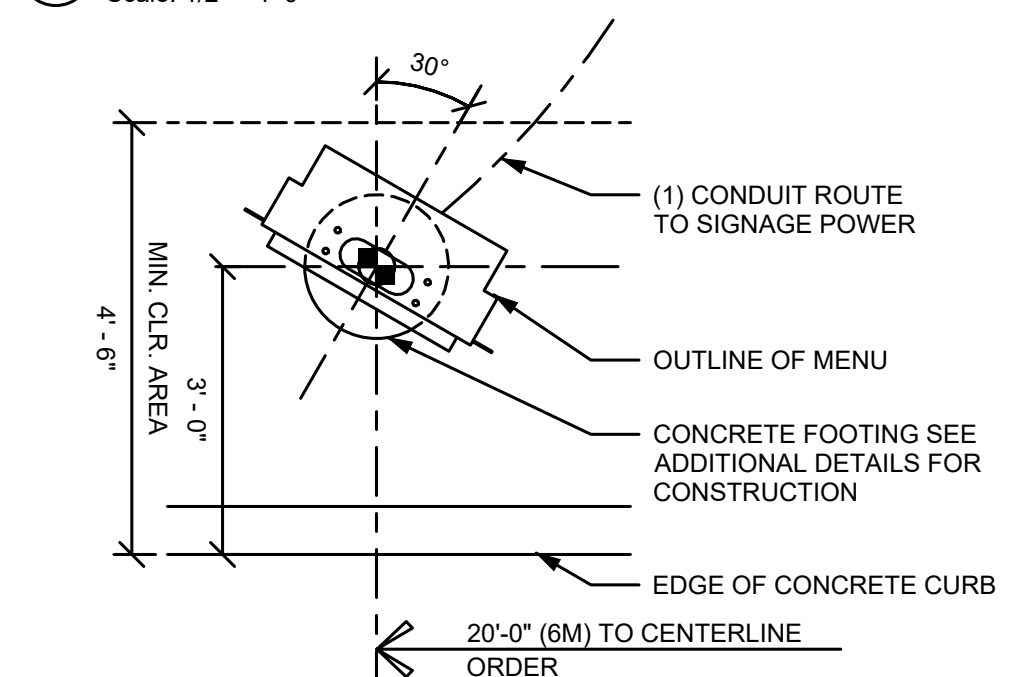
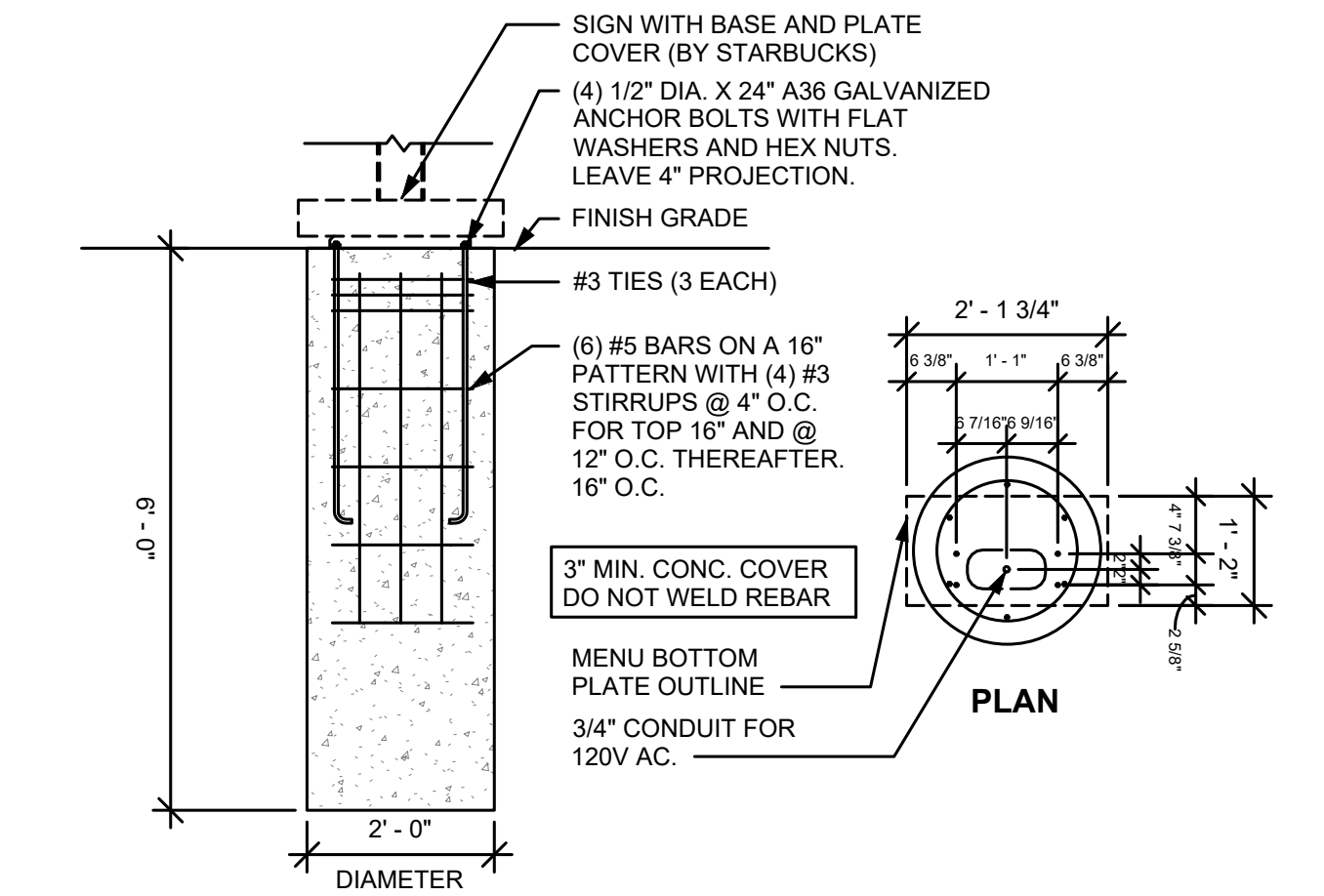
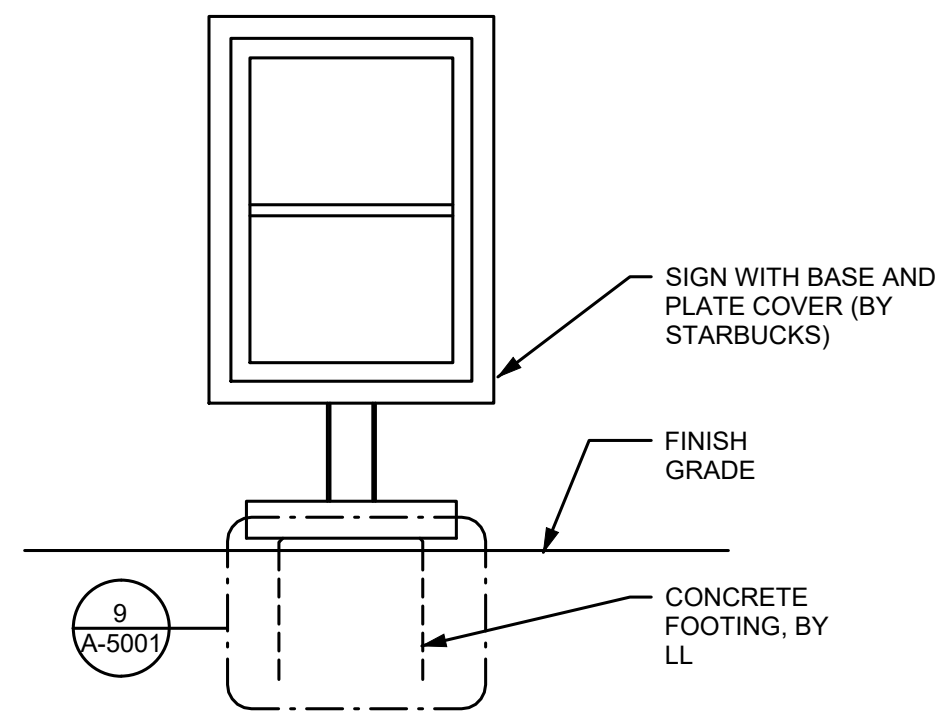
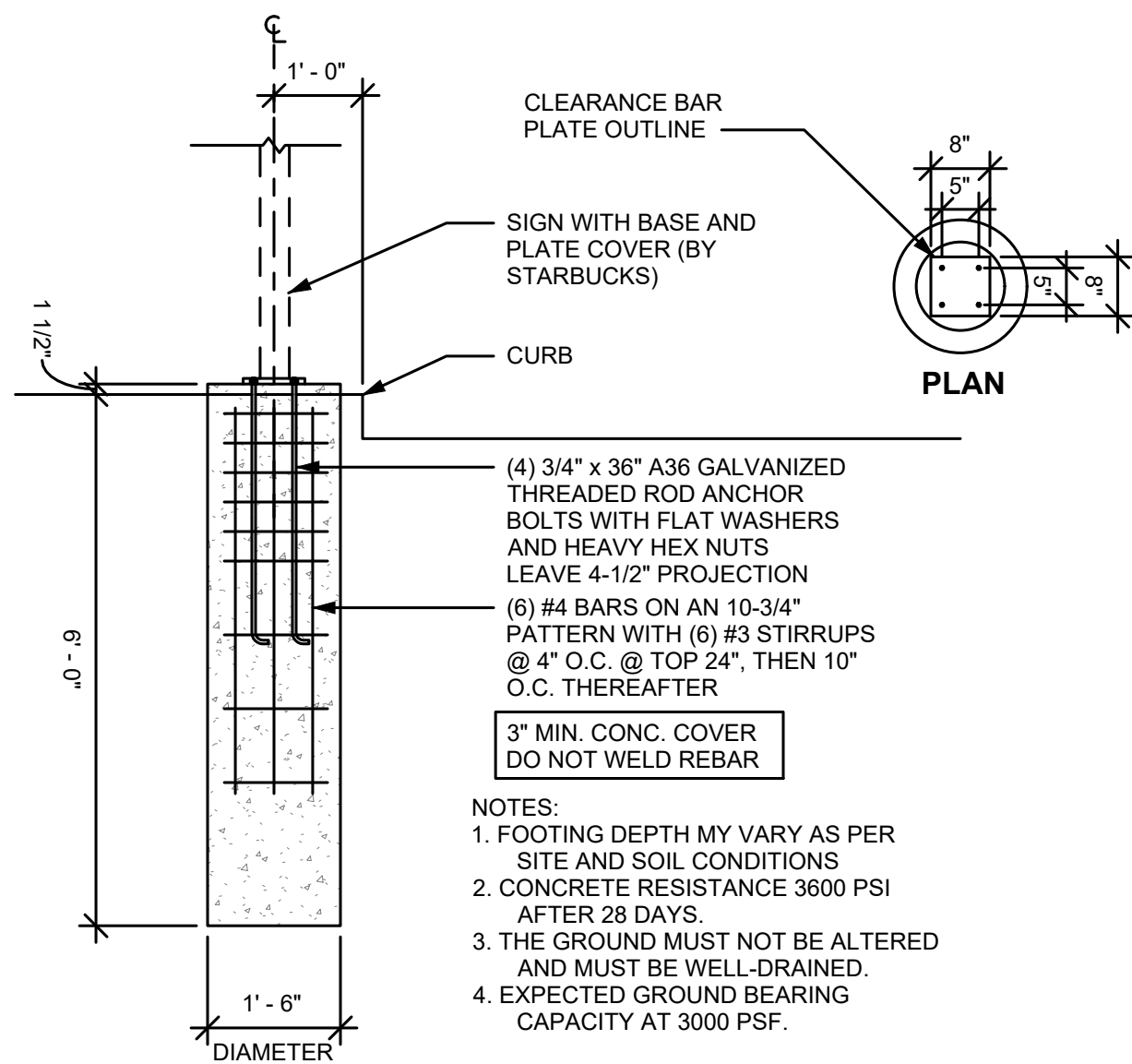
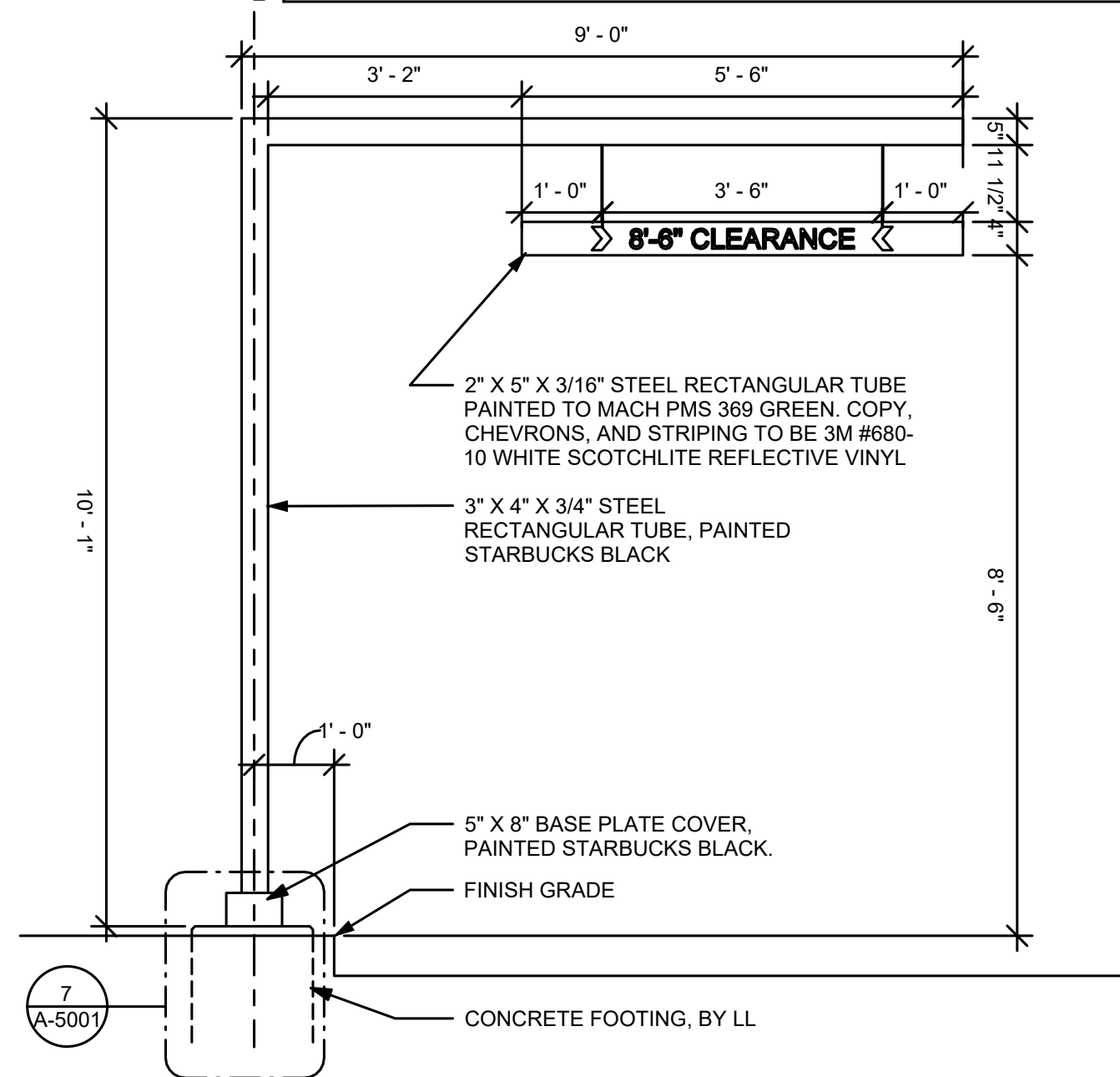
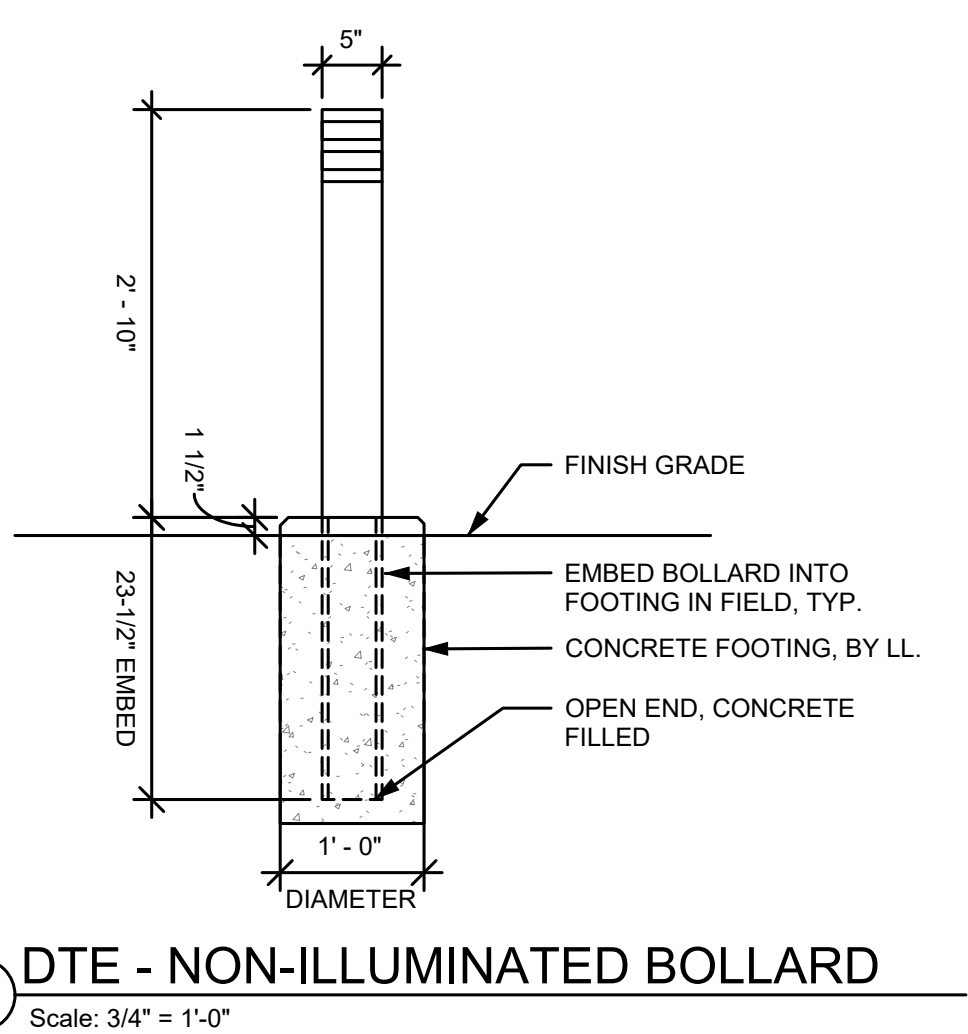
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SHEET TITLE:

C640



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REVISION



Starbucks
Franklin Gateway Development
Franklin, Indiana
General Sitework Details

Job No.:	Date Stamped:
17023	01/21/2019
Drawn By:	Checked By:
caw	tab
CAD FILE:	
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SHEET TITLE:
C650

GENERAL: WHEREVER A CONFLICT OR DEFICIENCY OCCURS BETWEEN THE CONSTRUCTION STANDARDS AND SPECIFICATIONS OF THIS IMMEDIATE PROJECT AND THE CONSTRUCTION STANDARDS AND SPECIFICATIONS ADOPTED BY THE LOCAL GOVERNMENT AGENCIES, THE HIGHER OR MORE RESTRICTIVE STANDARD OR SPECIFICATION SHALL APPLY.

A. STANDARD CONDITIONS

- The Contractor shall be responsible for obtaining or verifying that all permits and approvals are obtained from the respective city, county and state agencies prior to starting construction.
- It shall be the responsibility of each utility, contractor and/or sub-contractor to obtain all permits and approvals to open cut any road from the appropriate department.
- It shall be the Contractor's responsibility to determine the exact location of all existing utilities in the vicinity of the construction area prior to starting construction.
- It shall be the Contractor's responsibility for notification and coordination of all construction with the respective utility companies.
- It shall be the responsibility of the Developer and Contractor to maintain quality control throughout the project; failure to do so may result in removal and replacement of the defective work. It is recommended that the Developer have a qualified inspector on the job site at all times during construction.
- It is essential that the work be done in conjunction with this project shall be installed according to these specifications. The engineer will be required to certify to certain portions of this project upon completion. Therefore, it is necessary to obtain approval and acceptance by the city, county, state or government agency that construction was done in compliance with these plans and specifications.

B. CLEARING AND GRUBBING

- Clearing and grubbing shall consist of cutting removal and satisfactory disposal of all trees, down timber, brush, projecting roots, stumps, rubbish, boulders, broken concrete, fencing (as designated), and other material on the project site and within the boundary as shown on the construction documents and/or as designed by 'construction limits'.
- Special instructions shall be taken to insure that trees to be left remaining in the project area shall not receive limb, bark or root injuries. When such injuries occur, all rough edges of scarred areas shall be removed in accordance with accepted horticultural practice and the scars coated thoroughly with an asphaltum base tree paint.
- All "unsuitable material" from clearing operations stored in items B-1 shall be removed to disposal area(s) off of the project site; unless a "bury pit" shall be utilized in an area where it shall not be beneath building areas and/or pavement areas and shall not be located in an area where storm drainage structures shall be located or where impoundment of surface drainage may occur.
- Material shall not be disposed of by burying unless approved by the local and state fire marshal.

C. TREE REMOVAL AND PROTECTION

- Trees should only be cleared only where the area is to be occupied by road and surfaced areas in accordance with specifications of the city, county, state or government agency.
- Trees shall be removed from the project site or buried within the project as directed by the Developer and only in the areas designated.
- Trees shall be removed from the project where they interfere directly with the placement of storm or sanitary sewers and that such excavation is or will be fatal to such adjacent trees.
- The Contractor shall endeavor to save and protect trees of value and work which do not impair construction of improvements as designated. In the event cut or fill exceeds 0.5 foot over the root area, the Developer shall be consulted with respect to protective measures to be taken, if any, to preserve such trees.
- The contractor shall be responsible for determining the method for protection of tops, trunks and roots of existing trees on the project site that are to remain. Existing trees subject to construction damage shall be boxed, fenced or otherwise protected before any adjacent work is started. Earth or material or equipment shall not be stockpiled or stored within the spread of branches. Branches which need to be removed or are broken shall be neatly trimmed and shall be covered with tree paint.

D. STRIPPING OF TOPSOIL

- The contractor shall verify that all topsoil has been removed in the areas to be occupied by roads, walks and designated building areas. Topsoil shall be removed to a depth of six (6) inches or deeper, if necessary, to remove vegetable matter where required.
- Topsoil shall be kept separated from suitable fill materials and shall not be used to fill under pavement and/or building areas. Operations. Excess topsoil shall be spoiled on the site as directed by Owner or Engineer.
- Topsoil placed on the lot shall be reasonably free from subsoil debris and stones.

E. GRADING

- The contractor shall perform all grading operations to bring subgrades, after final compaction, to the required grades and sections of site improvements.
- Subgrade shall be proofrolled with suitable equipment and all spongy and otherwise unsuitable material shall be removed and replaced with suitable material.
- Subgrade shall be prepared in compliance with I.N.D.O.T. Standard Specifications, 1993 Edition - Section 207, as revised, with the city, state or government agencies approving the subgrade before paving.
- All fill materials shall be formed from soil free of deleterious material. Prior to placement of fill a sample of the proposed fill material should be submitted to the soils Engineer for his approval.
- All fill material in areas outside of building and pavement areas shall be compacted lightly and protected from erosion by one or more of the methods of item C, areas for building and pavement construction shall not have unsuitable material placed in the location and fill shall be compacted to 95% Standard Proctor. These areas shall be determined by the Developer's representative.

F. SANITARY SEWERS

- GENERAL
 - Current city, state or government specifications shall prevail as to materials and methods of construction.
 - The contractor shall be responsible for obtaining or verifying all permits for all of portions of this project prior to starting construction.
 - Sanitary sewers shall be installed in accordance with the Indiana Department of Environmental Management (Division of Water Management).
 - None of the sanitary sewers are to be constructed until a letter of approval from the Department of Environmental Management has been received by the Engineer. The Engineer will then contact the Developer or contractor of the said approval.
- GRAVITY SANITARY SEWERS
 - The Department of public works currently allows the use of the following pipe materials meeting or exceeding the minimum requirements/specifications set forth herein for the construction of gravity sanitary sewers:
 - Polyvinyl Chloride Pipe (PVC)
 - Ductile Iron Pipe (DIP)
 - Vitrified Clay Pipe (VCP) is NOT an approved material for the construction of sanitary sewers.

F. (CONTINUED)

- In general, all gravity sanitary sewer pipe shall be the bell and spigot type with elastomeric seal joints and smooth interior walls meeting or exceeding all requirements set forth in the latest ASTM standard reference herein.
- THE DEPARTMENT DOES NOT ALLOW THE USE OF SOLVENT CEMENT JOINT FOR GRAVITY SANITARY SEWERS EIGHT (8) INCHES IN DIAMETER OR LARGER.
- Each length of pipe shall be marked per the requirements of the respective ASTM Standards.
- GRAVITY SANITARY SEWERS MATERIALS
 - Polyvinyl Chloride Pipe
 - Pipe: Polyvinyl Chloride (PVC) gravity sanitary sewer pipe shall be the integral wall bell and spigot type with elastomeric seal joints and smooth interior walls meeting or exceeding all the requirements set forth on ASTM F-679 for pipe diameters greater than 15 inches.
 - For diameters 15 inches or less, the pipe shall have a minimum cell classification of 12454-B or 12454-C and for diameters greater than 15 inches, the pipe shall have a minimum cell classification 12454-C; with all pipe having a minimum tensile strength of 3450 MPA as defined in ASTM D-1784.
 - PVC sanitary sewer pipe shall have a minimum stiffness of 46 PSI for each diameter when measured at 5% vertical ring deflection and tested in accordance with ASTM D-2412.
 - Polyvinyl Chloride (PVC) ribbed sewer pipe meeting or exceeding requirements set forth in ASTM F-949-86a or ASTM F-794 is acceptable. The minimum cell classification acceptable shall be 12454-B or 12454-C as defined in ASTM D-1784. PVC ribbed sewer pipe shall have a minimum pipe stiffness of 50 psi when measured in accordance with ASTM D-2412.
 - Design: The minimum wall thickness for PVC sewer pipe and lateral sewer pipe 15" inches or less in diameter shall conform to SDR-35 Type PSM as specified in ASTM D-3034. The minimum wall thickness for PVC sewer pipe greater than 15" inches in diameter shall conform to T-1 as specified in ASTM F-679.
 - Marking: the date of manufacture, class of pipe, specification designation, size of pipe, name or trademark of manufacturer, and identification of plant location shall be legible marked on the outside of each pipe section in accordance with the ASTM D-3034.
 - Certification: The contractor upon request shall furnish the Department with manufacturer's certification stating that the pipe supplied meets or exceeds all requirements of the applicable ASTM standards and these standards.
- Joints
 - Flexible gasketed joints shall be compression type so that when assembled, the gasket inside the bell will be compressed radially on the pipe spigot to form a watertight seal. The assembly of joints shall be in accordance with pipe manufacturer's recommendations and ASTM D-3212. The gasket sealing the joint shall be made of rubber; of special composition having a texture to assure a watertight and permanent seal, and shall be the product of a manufacturer having at least (5) five years experience in the manufacture of rubber gaskets for pipe joints. The gasket shall be a continuous ring of flexible joint rubber of a composition and texture which is resistant to common ingredients of sewage, industrial wastes and groundwater, and which endure permanently under the conditions likely to be imposed by this service. The gasket shall conform the requirements of ASTM F-477.
 - NO SOLVENT CEMENT JOINTS SHALL BE ALLOWED.
- Fittings
 - Only manufactured fittings made of PVC plastic having a cell classification of 12454-B or 12454-C as defined in ASTM D-1784 shall be used.
 - Saddle connections shall not be allowed for new construction.
- DUCTILE IRON PIPE
 - Material: Ductile iron pipe in diameters from eight (8) inches through thirty six inches (36) shall be centrifugally cast and shall conform to ANSI Specifications A-21.51 and AWWA C-151 latest revision. Ductile iron pipe shall be class 50, 51, 52 or 54 wall thickness dependent upon site conditions and provided in minimum laying lengths of eighteen (18) feet. Ductile iron pipe larger than thirty six (36) feet in diameter shall be approved on a case by case basis by the department.
 - Fittings: Fittings shall be standardized for type the pipe of pipe and joint specified and shall comply with ANSI A-21.10, AWWA C-110.
 - Joints: Mechanical joints, slip or flanged joints shall be provided.
 - Weights and Marking: Weights of pipe fittings shall conform strictly to requirements of ANSI Specifications. The class designations for the various classes of pipe and fittings shall be cast onto the fittings in raised numerals, and cast or stamped on the outside of each joint of pipe. Weights shall be plainly and conspicuously pointed in white on the outside of each joint of pipe and each fitting after the exterior coating has hardened.
- Bedding and Backfill-Sanitary SewersThe following section provides the minimum requirements for the bedding of pipe and the backfilling of the trench.
 - Bedding-Sanitary Sewers
 - Trenching where the bottom of the trench is of undesirable material, an additional six(6) inches of trench bottom shall be excavated and a stable foundation shall be constructed using compacted No. 2 crushed stone.
 - All sanitary sewer pipe shall be laid to the lines and grade shown on the approved design plans unless otherwise approved by the department.
 - Bedding material shall be compacted No. 8 crushed stone or No. 8 fractured face aggregate and shall be placed the trench bottom such that after the pipe has been placed thereon, imbedded to grade and aligned, there remains a 4-inch minimum depth of material below the pipe barrel and a minimum of 3-inches below the bell.
 - The bell holes shall be excavated so that the entire pipe barrel rests on the bedding. The following presents the bedding requirements for each pipe classification:
 - Flexible Pipe: PVC Pipe
 - No. 8 crushed stone or No. 8 fractured face aggregate shall be placed around the sides of the pipe up to the sides of the pipe to the springline(1/2 the outside diameter). The material shall be shovel sliced or otherwise carefully placed and "walked" or hand tamped in to ensure compaction of the haunch area and complete filling of all voids. From the springline to twelve(12) inches above the crown of the pipe, bedding shall be added in six(6) lifts "walked" in for compaction. Backfilling of the remainder of the trench shall be as specified in the section
 - Semi-Rigid Pipe: Ductile Iron Pipe
 - No. 8 crushed stone or No. 8 fractured face aggregate shall be placed around the sides of the pipe up to the springline(1/2 the outside diameter). This material shall be shovel sliced or otherwise carefully place and "walked" or hand tamped in to ensure compaction of the haunch area and complete filling of all voids.
 - From springline to six(6) inches, of 1/2 the outside diameter above the top of the pipe, whichever is larger, bedding shall be added in six(6) lifts "walked" in for compaction.
 - Backfilling of the remainder of the trench shall be a specified later in the section.
 - Backfilling Sanitary Sewers
 - Backfill Materials
 - The following materials shall be used to backfill the trenches in accordance with and in the manner indicated by the requirements specified herein:

F. (CONTINUED)

- Class 1 Angular, six(6) to forty(40) millimeters(1/4 to 1 1/2 inch) graded stone such as crushed stone.
- Class 2 Course sands and gravel with maximum particle size forth (40) millimeters (1 1/2 inch), including variously graded sands and gravel containing small percentage of fines, generally granular and non-cohesive, either wet or dry. Soil types GW, GP, SW and SP are included in this class.
- Class 3 Fine sand and clay gravel,including sands, sand-clay mixtures, sand-clay mixtures, Soil types GM, GC, SM and SC are included in this class.
- Class 4 Silt, silty clays and clays, including silts of medium to high plasticity and liquid limits. Soil types MH, ML, CH, and CL are included in this class. These materials are not recommended for bedding.
- Backfill Around Pipe (bedding): Bedding and backfill materials shall be agreed upon prior to construction by the Engineer and the Contractor. Sample will be obtained and kept at the Engineer's office. No significant deviation from this material will be permitted for use without authorization by the Engineer and the Department.
- The term "select fill" shall mean the use of Class 2 or 3 backfill materials as described above.
- Areas Subject to Vehicular Traffic
 - In areas under proposed or existing paved roads or within five feet of pavement, sidewalks, curbs, gutters or other structures, granular backfill material complying with the requirements of the Indiana Department of Transportation Standard Specifications, 1993 Edition, as revised, shall be used.
 - The material shall be placed in uniform layers not six(6) inches, loose measurement. Within three(3) feet of the sanitary sewer pipe the backfill material shall be thoroughly and uniformly compacted with hand held mechanical tampers. The remaining backfill material shall be compacted with mechanical tampers. A minimum compaction of 95% percent Standard Proctor Density shall be achieved within the backfill material.
 - Jetting or flooding of the backfill or other alternative compaction methods and materials shall not be used without the approval of the city Department of Sanitation or Indiana Department of Transportation, dependent upon jurisdictional authority.
- Workmanship
 - Laying of Pipe in Cold Weather
 - The Engineer reserves the right to order pipe installation discontinued whenever, in our opinion, there is danger of the quality of work being impaired because of cold weather. The Contractor shall be responsible for heating the pipe jointing material so as to prevent freezing of joints.
 - Do not lay any pipe on frozen ground. No flexible or semi-rigid pipe shall be laid when the air temperature is less than 32 F unless proper precautions per the manufacturer's recommendations are taken by the Contractor and the method is approved by the Engineer and Department.
 - When pipes with rubber gaskets or resilient-type are to be laid in cold weather, sufficiently warm the gasket or joint material so as to facilitate making a proper joint.
 - Abandoned Sewers
 - Sewers and storm water drains which are to be abandoned shall be bulkheaded with mortar and an eight(8) inch thick brick wall. Sewers, storm water drains, and sewer structures which are to be abandoned in place shall be filled with Sand or Cellular Concrete and plugged, unless otherwise indicated on the Plans.
 - Service shall be maintained in such sewers and drains until the Department shall order bulkheads placed. No timber bulkheads shall be allowed. All castings on such abandoned structures are the property of the Department and shall be salvaged by the Contractor and delivered as directed. Unless otherwise specified, all abandoned manholes, catch basins, and inlets shall be removed to a depth of three(3) feet below the proposed or established grade or existing street grade, whichever is lower.
 - Dewatering and Control of Surface Water:
 - Where groundwater is encountered, the Contractor shall make every effort necessary to secure a dry trench bottom before laying pipe. The Contractor shall provide, install and operate sufficient trenches, pumps, pumps, hose, piping, wellpoints, etc., necessary to depress and maintain the groundwater level below the base of the excavation. If the Contractor is unable to remove the standing water in the trench, the Contractor shall over-excavate the proposed bottom grade of the sewer, bedding and place at least three(3) feet of class No. 2 crushed stone (Indiana Department of Transportation aggregate classification) in the over-excavated area.
 - The Contractor and/or Owner shall be liable for all lawsuits which may arise as a result of the Contractor's dewatering efforts.
 - The Contractor shall keep the site free of surface water at all times and shall install drainage ditches, dikes, pumps and perform other work necessary to divert or remove surface water and/or accumulation of surface water and/or groundwater shall be performed in a manner which will prevent the accumulation of water within the construction area.
 - UNDER NO CIRCUMSTANCES SHALL SURFACE WATER AND/OR GROUNDWATER BE DISCHARGED TO , DISPOSED OF, OR ALLOWED TO FLOW INTO THE CITY'S SANITARY SEWER SYSTEM.
 - Trenching
 - The width of the trench at and below the top of the sanitary sewer shall be only as wide as is necessary for proper installation and backfilling, and consistent with safety requirements. The minimum width of trench for sanitary sewers, including force mains, 42-inches in diameter and less shall be 1.25 times the outside diameter (O.D.) plus 12-inches:

Minimum Trench Width (inches)= 1.25 (O.D.) +12

The minimum trench width for sanitary sewers larger than 42-inches in diameter shall be determined on a case by case basis by the Engineer and approved by the department.
 - The design plans and specifications submitted to the Department for review approval and issuance of a construction permit shall include a detailed trench drawing.
 - The design of the sewer pipe and structures is predicated upon the width of trench indicated above and, should these limits be exceeded, the Contractor shall be responsible for the provision and installation of such remedial measures as may be required by the Engineer and/or the Department.
 - Bell holes shall be excavated for bell and spigot pipe mechanical joint pipe, so that the entire barrel of the pipe shall rest on the bedding.
 - The pipe trench shall not be excavated more than one hundred(100) feet in advance of pipe laying.
 - Whenever pipe trenches are excavated below the designed bedding bottom, the Contractor shall fill the over-excavated with mechanically compacted No. 8 (1/4 inch to 3/4 inch) crushed stone or No. 8 fractured face aggregate.
 - All rock, boulders and stones 6-inches in diameter and larger encountered in trenches shall be removed.
 - Boulders or rocks are not to be used for trench backfill.
 - In cases where material is deposited along open trenches, the material shall be placed so that no damage will result to the work or adjacent property as a result of rain or other surface wash.

F. (CONTINUED)

- If the bottom of the trench is of undesirable material, an additional six(6) inches of trench bottom shall be excavated and a stable foundation shall be constructed using compacted using a hand held mechanical tamper. Where the distance to stable ground is excessive, the Engineer shall order in writing other types of foundation as he deems necessary subject to the approval of the Department.
- Remove any rock(s) encountered within six(6) inches below the bottom of the trench and replace with No. 8 crushed stone or No. 8 fractured face aggregate and compact.
- Trench Box Pulling and Sheeting
 - When required by the Occupational Safety and Health Act (OSHA) to protect life, property, or the work, sheet and brace all open cut trenches in accordance with CFR 1926. Upon completion of the work, all temporary forms, shores, and bracing shall be removed, all vacancies or voids by the sheeting while being withdrawn, shall be carefully filled with bedding material.
 - The Contractor shall employ adequate safeguards to prevent movement of the pipe joint. If any movement should occur, the Contractor shall reinstall the pipe.
 - Any damage to pavement or other structures due to sheeting , shoring, or bracing shall be repaired by the Contractor at his own expense.
 - Sheeting and bracing which is to remain place shall be cut off at the elevation of 1.5 feet above the top of the sewer pipe.
- Line and Grade
 - The Contractor shall furnish and set all line and grade stakes (HUB) and stakes for bench marks. The bench marks shall be set in strategic locations of the project in order to facilitate the Contractor's installation of the line and grade stakes (HUB) for each pipeline. Only the laser method shall be used to set the grade of the pipeline. Any other method must first be approved in writing by the Department. The Contractor shall constantly check line and grade of the laser beam and the pipe.
 - Installation of Sanitary Sewers: Suitable tools and equipment shall be used for the safe and convenient handling and laying of pipe. Great care shall be taken to prevent pipe coating or wrappings from being damaged. Carefully examine all pipe for cracks and other defects.
 - No pipe or fittings shall be laid which are know to be defective. If pipe or fittings are discovered to be cracked, broken or defective after being laid, they shall be removed and replaced with ground material.
 - Thoroughly clean all pipe and fittings before installation. All pipe and appurtenances should be kept clean until accepted and completed work.
 - All field-cutting shall be done in a neat, trim manner using a hand or power saw and the cut end shall be beveled using a file or wheel to produce a smooth bevel of approximately 15" and be a minimum depth of one third the pipe wall thickness. Field cut pipe will only be allowed to be installed at manholes, at prefabricated trees and wyes, and at the connection of new sanitary sewer to existing sanitary sewer.
 - NOTE: Only smooth exterior pipe shall be used at manhole connections.
- Point of Commencement and Direction of Laying
 - The point of commencement for laying of sewer pipe shall be the lowest point in the proposed sewer line. Lay the pipe with the bell end of bell and spigot pipe or with the receiving groove end of tongue and groove pipe pointing upgrade. Any other procedure shall be followed only with permission of the Department.
 - Lay each pipe on an even firm bed as specified so that no uneven stress comes to any part of the pipe. Particular care shall be exercised to prevent the pipes from bearing on the sockets. Dig all bell holes for bell and spigot pipe.
 - Completely shove home all pipe (to the assembly mark.) On pipe of the tongue prove type thirty(30) inches and larger in diameter, pressure must be applied to the center of each pipe as it is laid by a winch and cable or other mechanical means.
 - All connection fittings shall be sealed with watertight stopper.
 - The Contractor shall extend the building eye lateral to the right-of-way and shall place a one(1) inch cast iron locator rod or magnetic locator tape above the end of the pipe to within three(3) feet of the ground surface. The purpose is to provide for ease of location of the wye stub.
- Construction Bulkheads
 - Before extending a sanitary sewer, the Contractor shall provide a watertight bulkhead in the existing sewer immediately down stream of the point of connection. This bulkhead shall be left in place until the new sanitary sewer has been cleaned of all accumulated water and debris and accepted by the Department.
 - During all intermissions in construction of the sanitary sewer pipe, the open face of the last pipe laid shall be plugged, covered or bulkhead so as to prevent sand, water, earth or other materials from entering the pipe.
 - Whenever pipe and special castings are required to be cut, the cutting shall be done by skilled workmen in a wach manner as to leave a smooth end at right angles to the axis of the pipe without damage to the pipe casting or cement lining. CUTTING TORCHES SHALL NOT BE USED.
- RELATIONSHIP TO WATER MAINS
 - Horizontal Separation
 - Sewers shall be laid at least 10 feet (3.0 m) horizontally from any existing or proposed water main. The distance shall be measured edge to edge. In cases where it is not practical to maintain a ten foot separation, the appropriate reviewing agency may allow deviation on a case-by-case basis, if supported by data from the design engineer such deviation may allow installation of the sewer closest to the water main, provided that the water that the water main is in a separate trench or in an undisturbed area. If the water main crosses under a sewer, adequate structural support shall be provided for the sewer to prevent damage to the water main.
 - Crossings
 - Sewer crossing water mains shall be laid to provide a minimum vertical distance of 18 inches (46 cm) between the outside of the water main and the outside of the sewer. If the water main crosses over the sewer, the sewer shall be above or below the sewer. The crossing shall be arranged so that the sewer joints will be equidistant and as far as possible from the water main joints. Where a water main crosses under a sewer, adequate structural support shall be provided for the sewer to prevent damage to the water main.
 - Special Conditions
 - when it is impossible to obtain proper horizontal and vertical separation is stipulated above, the sewer shall be designed and installed as a rigid pipe, and shall be pressure tested to assure watertightness prior to backfilling.
- Testing Gravity Sanitary Sewers
 - General
 - The contractor shall bear the complete cost and supply all equipment necessary to perform the test required.
 - All test shall be conducted under the observation of the Department's Observer. It shall be the Contractor's responsibility to schedule testing with the Observer.
 - Certifications: The Contractor upon request shall furnish the department with certified reports stating that inspection and specified test have been made and that the results thereof comply with the applicable ANSI Specifications and these standards for each.

F. (CONTINUED)

- Infiltration/Exfiltration Test
 - Once constructed, all sanitary sewers and manholes watertight and free from leakage. The rate of infiltration into the sanitary sewer system between any two adjacent manholes or the entire system shall not be in excess of 100 gallons per inch of pipe diameter per mile per day (100 gpd/inch mile). The Contractor shall be required to repair all visible leaks to satisfaction of the Department, even if the infiltration requirements are met.
 - Any leakage found during the infiltration test shall be corrected by the Contractor at his expense. The method of repair shall be per the approval of the Department; however, grouting of the joint or crack to repair the leakage shall not be permitted. If the defective portion of the sanitary sewer cannot be located, the Contractor shall remove and reconstruct as much of the work as is necessary to obtain a system that passes infiltration requirements.
- Deflection Test
 - All gravity sanitary sewers constructed of flexible pipe (PVC) shall be mandrel tested no sooner than thirty (30) days after installation. The deflection limit is 5% of the inside diameter of the pipe. Mandrel must be pulled through by hand with no mechanical device. Any failed section of pipe shall be reinstalled by the Contractor at no cost to the owner.
 - Low Pressure Air Test (Gravity Sewer)
 - All gravity sanitary sewers shall be tested for infiltration by means of a low pressure air test. A minimum of 5 p.s.i. must be maintained in the pipe for a period of no less than 5 minutes

REVISION



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General Notes

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